

**SOUTHERN CALIFORNIA'S
INTEGRATED WATER RESOURCES PLAN**

**VOLUME 3:
TECHNICAL APPENDICES**

Report No. 1107

January, 1996

VOLUME 3 - TECHNICAL APPENDICES

Purpose:

The purpose of Volume 3 is present the details of demands and supplies used for the technical analyses during the IRP process, as well as the technical description of the models and tools used.

Volume 3 is separated into 7 appendices:

Appendix A - Retail Water Demands

Appendix B - Local Project Data

Appendix C - Groundwater Conjunctive Use Storage Potential

Appendix D - State Water Project Supply Variation and Development Potential

Appendix E - MWD Capital Projects

Appendix F- IRPSIM Model Description

Appendix G - Supply Reliability and Least-Cost Planning

**SOUTHERN CALIFORNIA'S
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**APPENDIX A:
RETAIL WATER DEMANDS**

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APPENDIX A:

RETAIL WATER DEMANDS

Metropolitan uses the MWD-MAIN water demand forecasting model to project future urban water use for the region. MWD-MAIN is an econometric computer model that relates demographic and economic trends to residential, commercial, and industrial water demands. MWD-MAIN is a regionally calibrated version of the national IWR-MAIN model, developed for the U.S. Corps of Engineers, Institute for Water Resources. IWR-MAIN has gone through some major improvements which were jointly funded by the Federal Government, Metropolitan, the City of Phoenix, and the States of New York and Illinois. IWR-MAIN is considered to be state-of-the-art in demand forecasting and is currently used by district offices of the U.S. Corps of Engineers and U.S. Geological Survey, the Cities of Phoenix and Las Vegas, the States of New York and Illinois, and by some of Metropolitan's member agencies, including the City of Los Angeles and the San Diego County Water Authority.

Over the years, Metropolitan's water demand model has been reviewed during the Bay-Delta Hearings, Metropolitan's Blue-Ribbon Task Force, and the IRP. During these reviews, MWD-MAIN has been evaluated by experts from the University of California, University of Colorado, Johns Hopkins University, University of North Carolina, and Southern Illinois University. The reviewers found the model to be an acceptable and credible methodology for forecasting water demands in Metropolitan's service area. Where improvements could be made, they were incorporated into subsequent versions of the model and are reflected in the current forecast.

DEMOGRAPHIC/ECONOMIC DATA

MWD-MAIN uses projections of the following demographic and economic trends to project urban water use:

- | | |
|---|---|
| <input type="checkbox"/> Population | <input type="checkbox"/> Personal Income |
| <input type="checkbox"/> Housing by Type | <input type="checkbox"/> Price of Water/Sewer |
| <input type="checkbox"/> Employment by Category | <input type="checkbox"/> Climate |

The major sources of data include: (1) the Census Bureau; (2) California Department of Finance; (3) the California Employment Development Department; (4) the Bureau of Labor Statistics; (5) the National Oceanic Atmospheric Administration; (6) the Southern California Association of Governments; and (7) the San Diego Association of Governments. Metropolitan reviews this data to ensure accuracy and consistency. Table A-1 presents some of the key demographic data used to project regional demands for the SCAG region (Los Angeles, Orange, Riverside, San Bernardino, and Ventura Counties) and the SANDAG region (San Diego County).

**Table A-1
Demographic Data Provided by SCAG and SANDAG***

Demographic Data	1980 Census	1990 Census	2000 Projection	2010 Projection
<u>SCAG Region:</u>				
Population (millions)	10.20	12.35	14.08	15.86
Total Housing (millions)	3.68	4.15	4.64	5.25
Single-family (millions)	2.09	2.26	2.44	2.69
Multifamily (millions)	1.59	1.89	2.20	2.56
% Share of SF to Total	56.9%	54.3%	52.5%	51.2%
Persons per Household	2.78	2.97	3.04	3.02
Total Employment (millions)	5.10	6.18	7.04	8.18
Industrial (millions)	1.19	1.16	1.13	1.12
Commercial (millions)	3.91	5.02	5.91	7.06
<u>SANDAG Region:</u>				
Population (millions)	1.81	2.44	2.93	3.21
Total Housing (millions)	0.63	0.83	1.00	1.13
Single-family (millions)	0.41	0.52	0.62	0.68
Multifamily (millions)	0.22	0.31	0.38	0.45
% Share of SF to Total	65.2%	63.2%	61.7%	60.3%
Persons per Household	2.88	2.95	2.92	2.85
Total Employment (millions)	0.81	1.20	1.30	1.41
Industrial (millions)	0.11	0.14	0.15	0.15
Commercial (millions)	0.70	1.06	1.15	1.26
<u>Metropolitan's Service Area:</u>				
Population (millions)	12.01	14.79	17.01	19.07
Total Housing (millions)	4.30	4.98	5.64	6.37
Single-family (millions)	2.50	2.78	3.05	3.37
Multifamily (millions)	1.80	2.20	2.59	3.00
% Share of SF to Total	58.1%	55.8%	54.1%	52.8%
Persons per Household	2.79	2.97	3.02	2.99
Total Employment (millions)	5.91	7.38	8.34	9.59
Industrial (millions)	1.30	1.30	1.28	1.28
Commercial (millions)	4.61	6.08	7.06	8.31

* Based on draft growth management plans, originally developed in 1993.

Figure A-1 presents the projected population for Metropolitan's service area for three different SCAG/SANDAG forecasts. The prior two forecasts made by the regional governments fell short of actual population growth in the first three years. Figure A-2 presents the annual

population growth in the service area, showing the components of growth (natural increase and net migration).

Figure A-1
Population Forecasts for Metropolitan's Service Area

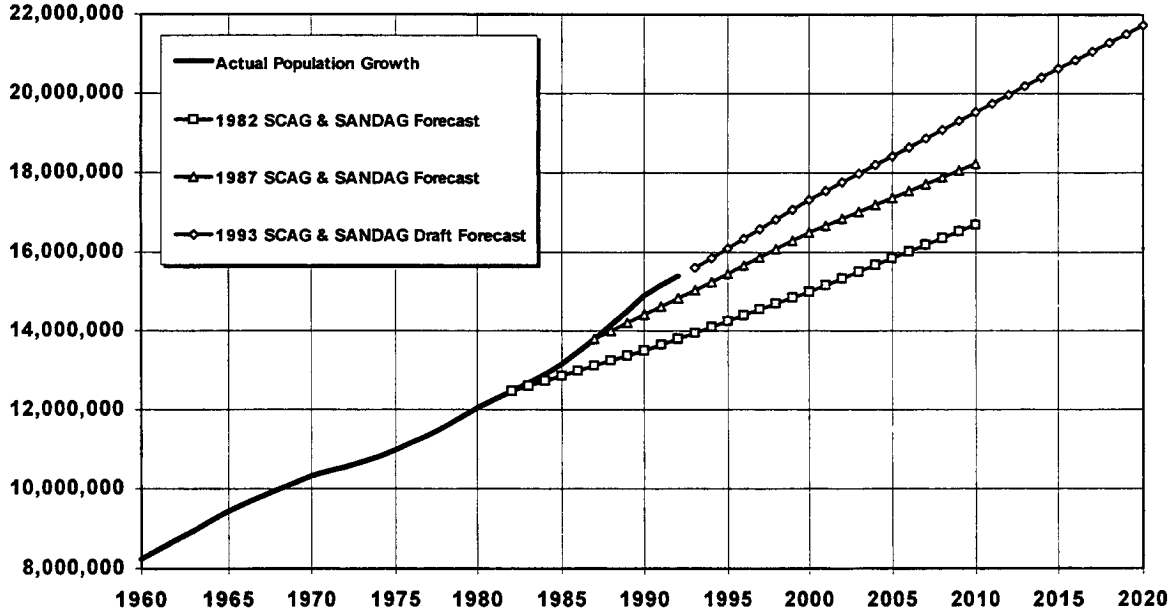
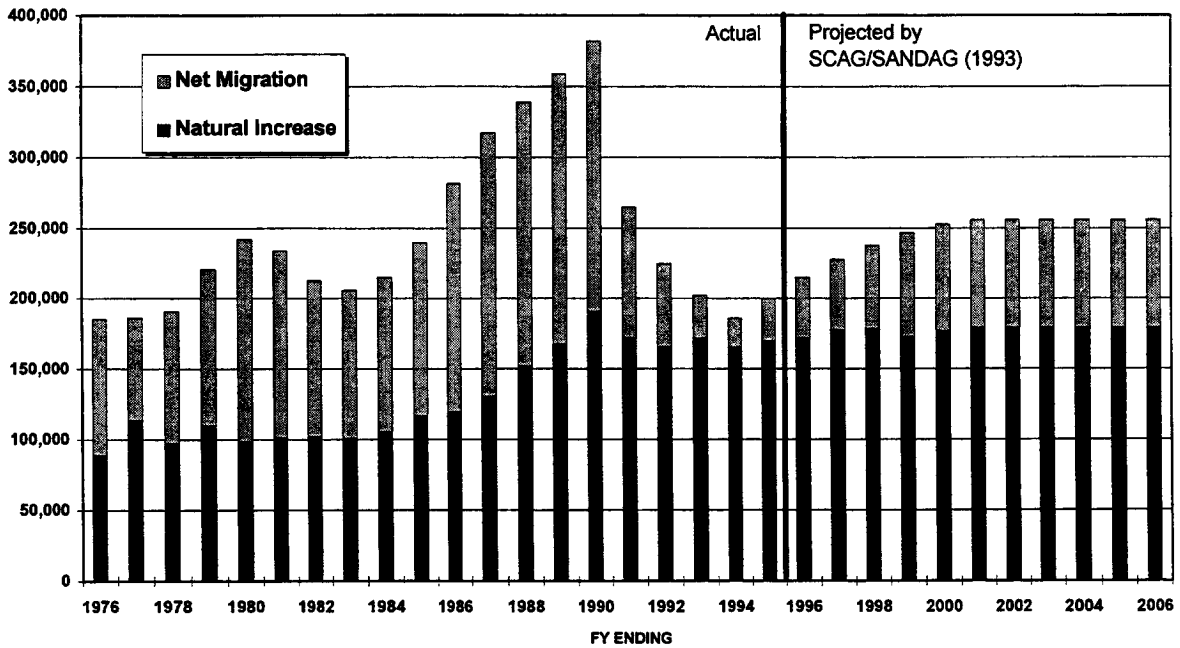


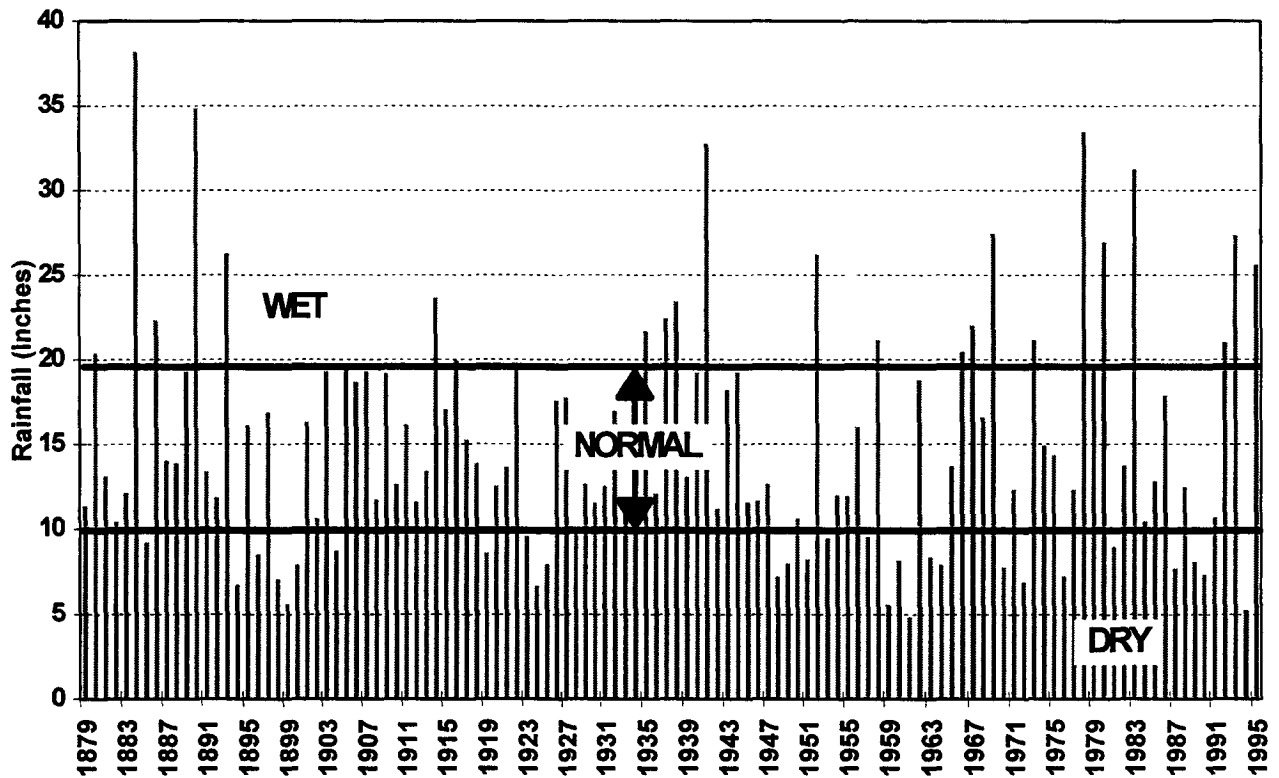
Figure A-2
Annual Population Growth in Metropolitan's Service Area



RAINFALL DATA

Local rainfall can impact Metropolitan's water sales in two ways. The first impact relates to retail water demands. When rainfall is heavy (wet conditions), retail water demands are low; and when rainfall is light (dry conditions), retail water demands are high. This is mainly due to landscape irrigation of residential yards and large public areas. The second impact relates to local supplies. When rainfall is heavy, local runoff is high -- naturally filling local reservoirs and groundwater basins; but when rainfall is low, local runoff is unable to naturally fill local storage -- thereby increasing Metropolitan's seasonal sales. Figure A-3 presents 117 years of Los Angeles civic center rainfall, from 1887 to 1995. Note that three of the last four years (1992, 1993, and 1995) had annual rainfall totals greater than 20 inches. This recent rainfall is one of the major reasons why current water sales are so low.

Figure A-3
Los Angeles Civic Center Rainfall



WATER AND SEWER PRICES

Based on ten years of retail water use data, demographic data, climate, and price of water and sewer service, price elasticity estimates were statistically derived. Price elasticity is a measurement of water customers' response to changes in the price of water. Generally, if the price of water goes up, it is expected that the quantity of water demanded will go down. Measuring price elasticity is very difficult because all of the other factors that could be

responsible for changes in historical water use (such as changes in population growth, economy, weather, and conservation) must be controlled for. Statistical regression analysis is used to parcel out the effect that changes in the price of water have on changes in water demand. Metropolitan's water demand consultants have estimated that the price elasticity for urban water use ranges from -0.13 to -0.27, depending on the season (winter or summer) and type of use (single-family, industrial, or commercial). The overall, weighted urban annual average price elasticity for Metropolitan's service area is about -0.22, meaning that a 10 percent real (above inflation) *increase* in price will lead to a 2.2 percent *decrease* in water use.

Based on the regional supply investments identified in the IRP Preferred Resource Mix, the average retail cost increase is about 4.5 percent per year. Discounting for the effects of inflation (estimated to be about 3 percent per year), yields a real increase in retail cost of about 1.5 percent per year. Therefore, after 10 years the real increase in the price of water is expected to be about 15 percent greater than it is today. The quantity of water at the retail level will, therefore, be about 3 percent lower than it would have been if prices remained constant (in real dollars).

URBAN PER CAPITA WATER USE

In reaction to the recent low water sales, the question of "what is the long-term trend in water demands, and has that trend changed recently" has been raised. To help answer that question, urban per capita water use can be examined. Per capita water use (dividing retail urban water use by population) can be useful when evaluating trends in water use only if the major factors that drive changes in per capita water use are known. MWD-MAIN does not use the per capita use approach to project water demands, but the model can summarize the resulting demand forecast in per capita use terms in order to help explain future trends.

Factors that cause per capita water use to increase include: (1) income -- the greater the income, the greater the landscaping requirements and indoor water using appliances; (2) commercial industry mix -- those commercial establishments that use more water, such as restaurants, hotels, and amusement/recreation, are expected to grow faster than those establishments that use less water; (3) commercial labor force -- the fraction of people employed in commercial activities is expected to increase, thereby increasing overall water use; and (4) inland growth -- the growth of people and jobs in the inland desert regions of the service area is going to be greater in the future, where water use is higher because of the hot and dry conditions. Factors that cause per capita water to decrease include: (1) housing mix -- multifamily housing, which uses less water than single-family housing, is expected to grow faster; (2) family size -- the average persons per household is expected to continue to increase until 2010 (when it starts to decline slightly), which causes per capita water use to decrease; (3) industrial industry mix -- those manufacturing activities that use more water, such as aerospace and defense related industries, are expected to decrease overtime; and (4) industrial labor force -- as time goes on, manufacturing jobs will be replaced by service oriented jobs (which use less water), thereby reducing overall urban water use.

Table A-2 presents a summary of actual and projected per capita water use from 1990 to year 2010. The table shows how per capita use, which is split into residential, commercial, industrial, and public/other, is expected to change in the future, and the factors responsible for that change. It should be noted that these per capita estimates do not include conservation. The effects that anticipated conservation has on reducing overall per capita water use is shown at the bottom of the table.

Table A-2
Changes in Per Capita Water Use
(assumes normal weather conditions)

	Base Per Capita			Factors Affecting Per Capita Use					
	Water Use (GPCD)			Changes in GPCD Between 1990 - 2010					
	1990	2010	Change	Income	Housing Mix	Family Size	Industry Mix	Labor Force	Inland Growth ¹
Residential	136.7	141.5	4.8	4.9	-3.3	-0.3	0.0	0.0	3.5
Commercial	38.9	43.8	4.9	0.0	0.0	0.0	2.3	0.5	2.1
Industrial	12.3	10.0	-2.3	0.0	0.0	0.0	-1.5	-1.9	1.1
Public/Other	18.1	19.7	1.6	0.0	0.0	0.0	0.0	0.0	1.6
Total	206.0	215.0	9.0	4.9	-3.3	-0.3	0.8	-1.4	8.3
With Conservation ²	206.0	190.0							

¹ Represents growth shifting from coastal areas to inland desert areas that have hotter & drier climates.

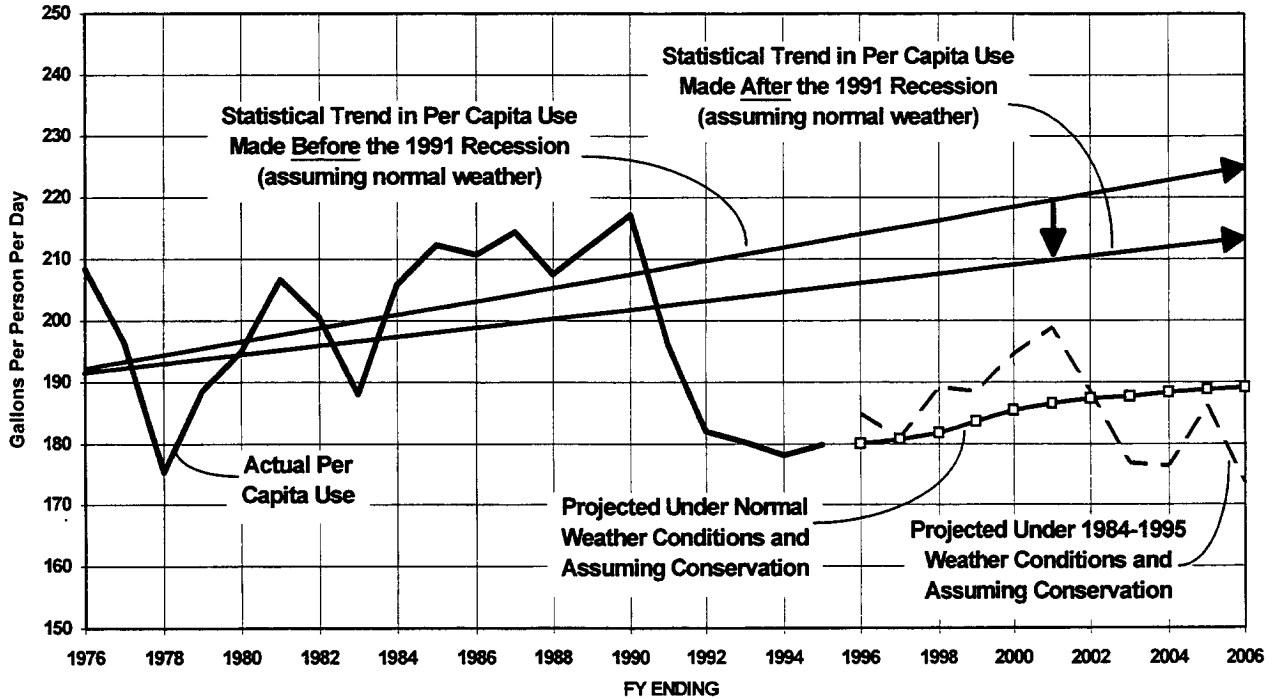
² Reflects new conservation (post 1990), including 1991 plumbing codes, plumbing retrofits, landscaping efficiency, commercial & industrial, leak detection/repair, and effects of retail water prices.

Table A-2 indicates that per capita water use is expected to increase from 206 gallons per person per day (GPCD) in 1990 to 215 GPCD by 2010. However, if planned conservation programs are fully implemented, then per capita water use will be about 190 GPCD, a reduction of about 12 percent.

Figure A-4 presents actual per capita water use from 1976 to 1995 and projected per capita use based on different statistical trends. During the 1977-78 period, per capita water use decreased from 210 GPCD to 175 GPCD, a 16.6 percent reduction over two years. This decrease was due to three factors: (1) mandatory conservation due to the 1976-77 drought; (2) an economic recession; and (3) three years of extreme wet weather. However, after these events “normalized,” per capita water use quickly increased to its pre-1977-78 levels. During 1983, local rainfall was one of the heaviest on record (over 32 inches) causing per capita use to decrease from 205 GPCD to about 188 GPCD. During the period from 1985 to 1990, the region experienced strong economic growth (annual population growth was over 300,000) and hot and dry weather. This caused per capita water use to remain over 210 GPCD. During the 1991-1992 period, per capita use decreased from 217 GPCD to about 181 GPCD, a 16.6 percent reduction over two years. The events that caused the significant decrease were

remarkably similar to those that caused per capita use to decrease back in 1978, namely drought related-conservation, an economic recession, and three years of extreme wet weather.

Figure A-4
Urban Per Capita Water Use in Metropolitan's Service Area



Based on the best data available before the 1991 economic recession, the statistical trend for long-term per capita water use (without conservation and under normal weather conditions) indicated that future per capita water use would be around 225 GPCD by year 2005. After the 1991 recession, many demographers and economists revised their long-term economic outlooks for California showing slower and more dense growth. Based on these new demographic and economic projections, Metropolitan staff made another demand forecast, reducing the long-term trend in per capita water use to about 212 GPCD by 2005. However, neither of these trends in per capita use accounted for conservation. Assuming full implementation of conservation BMPs, the long-term trend in per capita water use is expected to remain at about 190 GPCD. This is the demand trend staff has been projecting for the last three years and during the IRP process.

RETAIL DEMAND PROJECTIONS

Based on the SCAG/SANDAG demographic data and the trends in urban per capita water use, the projection of total regional demands are shown in Figure A-5. The demands are shown for three weather scenarios: (1) wet conditions; (2) normal conditions; and (3) dry conditions. In addition, demands under a repeat of 1984-1995 weather conditions is shown for illustrating how projected demands could vary year to year. Based on 70 different historical weather

traces, retail demands can vary as much as 500,000 acre-feet in any given year due to weather alone.

Figure A-5
Regional Retail Water Demands for Metropolitan's Service Area

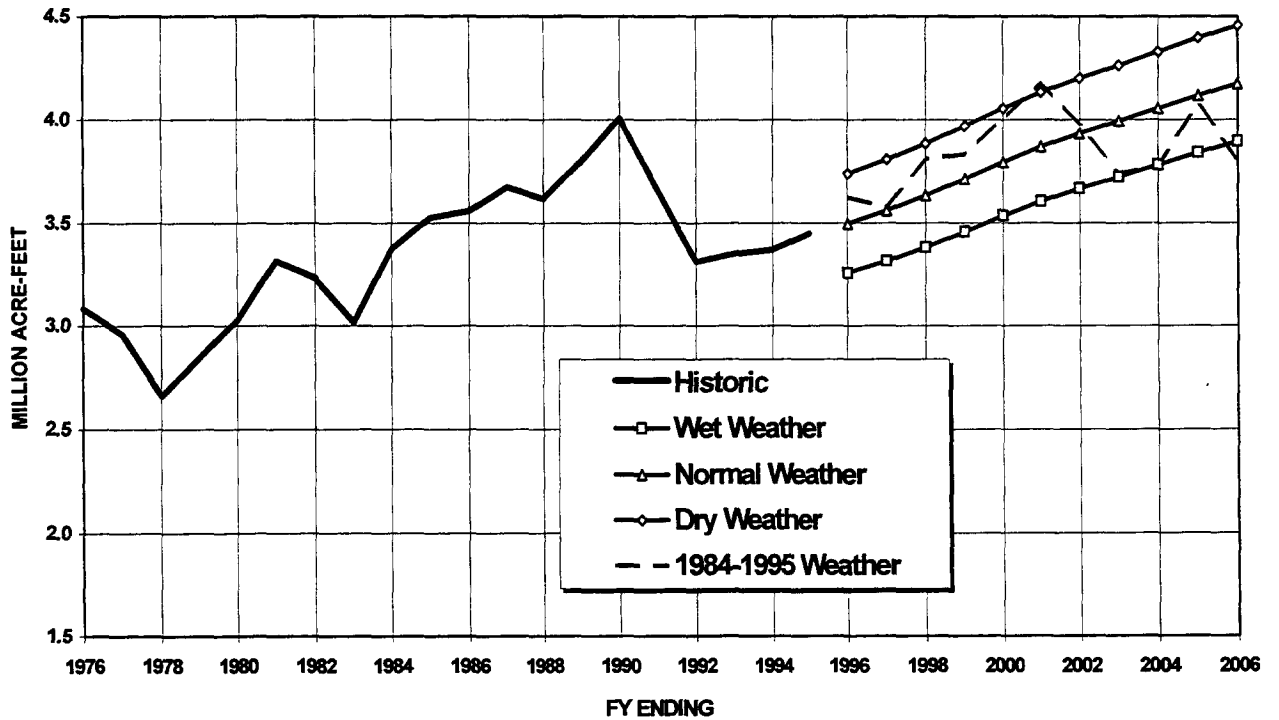


Table A-3 presents the population forecast by member agency. Table A-4 presents the M&I retail-level demand projections by member agency. Table A-5 presents the retail-level agricultural demands. The agricultural demands were projected based on current and future land use trends.

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**APPENDIX B:
LOCAL PROJECT DATA**

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LOCAL PROJECT DATA

During the IRP process, Metropolitan's member agencies and sub-agencies provided data to Metropolitan on local water recycling and groundwater recovery projects. The data included any projects that were already operational, under construction, or in some stage of design, planning, feasibility, or reconnaissance. The local project database currently consists of 159 reclamation projects and 38 groundwater recovery projects. Project information contained in the local project database include: on-line dates, supply yield, capital costs, interest rates, terms of debt, and O&M costs. The data was used to estimate annual total unit costs for each project through the year 2020. Table B-1 shows data on local water recycling projects. Table B-2 shows data on groundwater recovery projects.

Table B-1 (cont.)
LOCAL WATER RECYCLING PROJECTS

NAME	MEMBER AGENCY	PROJECT TYPE	PROJECT STATUS	FIRST YEAR OF YIELD	YIELD1995	YIELD2000	YIELD2010	YIELD2020	INTEREST	TERM	Annual Debt Service	OPCOST1995 (\$/AF)	OPCOST2000 (\$/AF)	OPCOST2010 (\$/AF)	ESTIMATED OPCOST2020 (\$/AF)	ULTIMATE TOTALCOST (\$/AF)
Branch Springs	SDCWA	D	D	1996	0	220	220	220	3.1	20	\$81,406	364	410	550	739	\$739
San Luis	SDCWA	D	D	1997	0	1,150	1,750	1,750	3.3	20	\$963,111	342	410	560	753	\$753
San Luis Phase B	SDCWA	D	D	1997	0	500	700	700	3.3	20	\$632,491	294	410	550	739	\$739
Chowchilla	SDCWA	D	R	1998	0	750	3,000	3,000	7.0	30	\$1,496,040	531	410	550	739	\$1,236
Poway	SDCWA	D	R	1998	0	500	2,000	2,000	7.0	30	\$1,152,831	486	410	550	739	\$1,316
Chico Water Program - Phase A (North City)	SDCWA	D	C	1998	0	5,000	14,500	14,500	7.0	30	\$19,932,536	193	410	550	739	\$2,114
Chico Water Program - Phase B	SDCWA	D	D	1998	0	250	850	850	7.0	25	\$0	363	410	550	739	\$739
San Luis	SDCWA	D	D	1998	0	500	750	750	1.6	20	\$419,166	363	410	560	753	\$753
San Luis Phase A	SDCWA	D	R	1999	0	2,800	2,800	2,800	5.1	25	\$2,463,252	711	410	560	753	\$1,632
Escondido	SDCWA	D	D	2000	0	100	400	400	6.1	25	\$643,524	711	410	560	753	\$2,361
Recon do Distrito	SDCWA	D	D	2000	0	150	300	300	7.0	30	\$134,541	152	410	550	739	\$1,188
Valley Center Phase B	SDCWA	D	P	2001	0	0	1,000	1,000	7.0	30	\$753,190	523	410	600	806	\$1,560
San Rafael Phase B	SDCWA	D	R	2001	0	1,200	1,200	1,200	7.0	30	\$702,977	200	410	560	753	\$1,338
Oceanside Phase B	SDCWA	D	R	2001	0	0	700	1,000	7.0	30	\$622,637	243	410	550	739	\$1,362
Yuba Phase B	SDCWA	D	R	2001	0	0	3,000	3,000	7.0	30	\$5,101,606	193	410	300	403	\$2,104
Chico Water Program - Phase B (North City)	SDCWA	D	P	2005	0	0	500	1,000	7.0	30	\$718,554	200	410	560	753	\$1,471
Oceanside Phase C	SDCWA	D	P	2005	0	0	1,000	1,000	7.0	30	\$1,053,880	294	410	560	753	\$1,806
San Marcos	SDCWA	D	P	2005	0	0	1,000	1,000	7.0	30	\$1,904,169	294	410	550	739	\$2,643
San Luis Phase C	SDCWA	D	R	2005	0	2,600	2,600	2,600	7.0	30	\$6,790,339	519	410	560	753	\$3,364
Chowchilla (San Jacinto) WRF - Phase B	SDCWA	D	P	2005	0	0	600	600	7.0	30	\$479,036	288	410	250	336	\$1,134
Fairwood Reclaimed Water Dist. - Phase B	SDCWA	D	R	2010	0	0	1,900	1,900	7.0	30	\$2,537,107	229	410	550	739	\$2,074
Encho Basin - Phase B	SDCWA	D	R	2020	0	0	4,000	4,000	7.0	30	\$19,352,596	193	410	300	403	\$5,241
Chico Water Program - Phase C	SDCWA	D	P	2020	0	0	0	0	7.0	30	\$0	141	172	281	378	\$378
Potrero Reclamation Project	These Valleys MWD	D	O	1966	9,000	9,400	9,600	9,600	0.0	0	\$0	253	329	484	650	\$650
City of Industry Reclamation System - Phase A	These Valleys MWD	D	O	1963	3,360	3,360	3,360	3,360	6.5	25	\$0	206	252	411	552	\$552
Wickiup Valley Reclamation Project	These Valleys MWD	D	O	1986	1,500	2,000	2,000	2,000	6.5	25	\$0	206	252	411	552	\$552
Wickiup Valley Reclamation Plant Expansion	These Valleys MWD	D	O	1996	0	500	500	500	7.0	25	\$600,674	206	252	411	552	\$1,754
Rowland Reclamation Project	These Valleys MWD	D	O	2000	0	2,000	2,000	2,000	7.0	25	\$818,645	188	329	484	650	\$1,060
West Branch Reclamation Project	These Valleys MWD	D	F	2000	0	6,600	6,600	6,600	7.0	25	\$1,023,306	222	329	484	650	\$806
West Branch Irrigation Extension	These Valleys MWD	D	O	1978	375	375	375	375	6.5	25	\$0	8	9	11	15	\$15
Colton County Club	Upper SCV MWD	D	O	1978	0	25,000	25,000	25,000	7.0	30	\$2,402,520	79	96	157	211	\$307
San Gabriel Valley Water Reclamation Project I	Upper SCV MWD	D	F	2000	0	6,000	10,000	10,000	7.0	30	\$961,008	79	96	157	211	\$307
San Gabriel Valley Water Reclamation Project II	Upper SCV MWD	D	F	2000	0	2,810	3,267	4,000	7.0	25	\$511,653	123	140	160	200	\$328
Punilla Hills Phase I	Upper SCV MWD	D	R	2000	850	15,000	20,000	20,000	5.4	25	\$3,532,222	209	256	417	560	\$560
West Branch Water Recycling Project - Phase I	West Branch MWD	D	C	1997	0	5,000	20,000	20,000	6.5	25	\$4,283,532	419	512	835	1122	\$1,336
West Coast Bank Project	West Branch MWD	D	R	1998	0	5,000	15,000	15,000	7.0	25	\$3,279,753	209	256	417	560	\$779
West Branch Water Recycling Project - Phase II	West Branch MWD	D	D	2000	0	5,000	15,000	15,000	7.0	25	\$2,558,268	222	329	417	560	\$731
West Branch Water Recycling Project - Phase III	West Branch MWD	D	R	2000	0	261	261	261	6.5	25	\$0	253	329	484	650	\$650
March Reclamation Project	West Branch MWD	D	O	1980	1,310	1,310	1,310	1,310	6.5	25	\$0	253	329	484	650	\$650
Indian Hill Reclamation Project	West Branch MWD	D	O	1984	360	360	672	672	6.5	25	\$0	253	329	484	650	\$650
Rancho California/Jacumba Branch Reclamation	West Branch MWD	D	O	1984	730	730	730	730	6.5	25	\$0	253	329	484	650	\$650
Elmore Valley/Redwood Canyon Reclamation	West Branch MWD	D	O	1984	110	224	560	560	6.5	25	\$0	253	329	484	650	\$650
Elmore Valley/Horse Hill Reclamation	West Branch MWD	D	O	1989	1	2	3	3	5	25	\$0	253	329	484	650	\$650
San Luis Water Reclamation Facility	West Branch MWD	D	O	1990	0	4,500	5,400	5,400	7.0	25	\$2,762,927	147	329	484	650	\$1,162
Elmore Valley Water Reclamation Project	West Branch MWD	D	P	2000	0	4,500	5,400	5,400	7.0	25	\$2,762,927	147	329	484	650	\$1,162

Project Type: D = Direct, R = Replenishment, B = Both
Project Status: O = Operational, C = Construction, D = Design, R = Reconnaissance, F = Feasibility

Table B-2

METROPOLITAN WATER DISTRICT OF SOUTHERN CALIFORNIA GROUNDWATER RECOVERY PROGRAM										
Cost Year: 1994		Project Background Data								
Project Name	Contaminant	Total Yield (af/yr)	MWD Replenishment (af/yr)	Est. Start Year	Estimated Capital (\$ millions)	Estimated Ann. Capital (\$1000/yr)	Estimated O&M Cost (\$1000/yr)	Estimated Repl. Cost (\$1000/yr)	Estimated Unit Cost (1994\$/af)	
APPROVED PROJECTS										
1	Santa Monica GW Treatment Plant	VOC	1,800	0	1993	\$2.9	\$300	\$371		\$373
2	Burbank Lake Street GAC Plant	VOC	2,744	2,744	1993	\$1.4	\$145	\$607	\$571	\$482
3	West Basin Desalter No. 1	TDS	1,524	0	1993	\$1.5	\$130	\$833		\$632
4	Oceanside Desalter No. 1	TDS	2,200	0	1994	\$5.8	\$595	\$888		\$674
5	Tustrn Desalter	TDS	3,271	909	1996	\$6.9	\$651	\$996	\$189	\$561
6	Irvine Desalter	TDS, VOC, Se	6,700	1,926	1998	\$28.5	\$2,197	\$2,832	\$401	\$810
7	Rowland GW Treatment Project	TCE/TDS	516	0	2000	\$2.3	\$191	\$216		\$787
8	Menifee Basin Desalter	TDS	3,360	0	1999	\$16.5	\$1,141	\$1,571		\$807
9	Chino/SAWPA Desalter No. 1	TDS/Nitrate	8,000	0	1998	\$41.5	\$3,349	\$2,200		\$694
APPROVED PROJECTS - Subtotal			30,115	5,579		\$107	\$8,700	\$10,513	Ave =	\$647
PROJECTS UNDER REVIEW										
10	Beverly Hills Desalter	TDS	2,688	0	1999	\$10.3	\$898	\$800		\$632
11	Arlington Desalter *	TDS/Nitrate	7,200	0	1998	\$23.4	\$1,727	\$2,310		\$561
12	Capistrano Beach Desalter	TDS	1,372	0	1999	\$4.2	\$352	\$389		\$540
13	San Juan Basin Desalter No. 1	TDS	2,200	0	1999	\$11.4	\$959	\$796		\$798
14	Baldwin Park Operable Unit	VOC	24,100	24,100	1999	\$18.1	\$1,878	\$3,907	\$5,013	\$448
15	Sweetwater Desalter No. 1	TDS	3,440	0	1998	\$8.3	\$1,214	\$1,092		\$670
PROJECTS UNDER REVIEW - Subtotal			41,000	24,100		75.7	7029.5	9293.6	Ave =	\$608
(Approved + Review Projects) TOTAL			71,115	29,679						
PROJECTS UNDER PLANNING										
16	Oceanside Desalter No. 2	TDS	3,360	0	1998	\$5.5	\$464	\$857		\$393
17	San Juan Basin Desalter No. 2	TDS	2,800	0	2000	\$13.0	\$1,097	\$826		\$687
PROJECTS UNDER PLANNING - Subtotal			6,160	0		\$19	\$1,561	\$1,683	Ave =	\$540
(Approved + Review + Planning Projects) TOTAL			77,275	29,679						
POSSIBLE PROJECTS										
18	San Pasqual Basin Desalter	TDS/Nitrate	5,000	0	2005	\$9.6	\$810	\$1,700		\$502
19	Winchester/Hemet Desalter	TDS	3,000	1,500	2001	\$12.5	\$1,055	\$1,300	\$312	\$889
20	Laguna Beach GW Treatment Project	Color	2,000	500	2001	\$6.3	\$532	\$336	\$104	\$486
21	Santee/EI Monte Basin Desalter	TDS	1,000	0	2001	\$2.7	\$230	\$455		\$685
22	Otay/Sweetwater Desalter	TDS	3,000	0	2002	\$8.9	\$753	\$1,155		\$636
23	Corona/Temescal Basin Desalter	TDS/Nitrate	10,000	0	2002	\$28.4	\$2,392	\$2,730		\$512
24	Perris Basin Desalter	TDS	6,000	0	2002	\$17.0	\$1,434	\$1,750		\$531
25	Chino/SAWPA Desalter No. 2	TDS/Nitrate	8,000	9,200	2002	\$33.1	\$2,311	\$2,010	\$1,914	\$779
26	Torrence Elm Ave. Fac.	Chloride	4,000	0	2004	\$3.7	\$312	\$2,081		\$598
27	Western/Bunker Basin Treatment Pro	Nitrate	8,100	0	2002	\$15.4	\$1,302	\$3,360		\$576
28	IRWD Colored Water Treatment Proj.	Color	10,000	2,625	2012	\$16.8	\$1,417	\$1,680	\$546	\$364
29	West Basin Desalter No. 2	TDS	6,000	0	2002	\$13.5	\$1,139	\$2,701		\$640
30	West Basin Desalter No. 3	TDS	5,000	0	2003	\$14.0	\$1,181	\$2,179		\$672
31	Tijuana River Valley Desalter	TDS	2,500	0	2004	\$5.3	\$443	\$1,107		\$620
32	San Dieguito Basin Desalter	TDS	5,000	0	2003	\$14.7	\$1,240	\$1,575		\$563
33	OCWD Undetermined Colored Water Projects	Color	12,000	3,000	2004	\$26.3	\$2,215	\$3,150	\$624	\$499
34	Rubidoux/Western Desalter	TDS/Nitrate	3,000	0	2004	\$8.9	\$753	\$1,155		\$636
35	Chino/SAWPA No. 3	TDS/Nitrate	9,050	10,400	2005	\$37.4	\$2,614	\$2,273	\$2,163	\$779
36	Hunt Beach Colored Water	Color	5,000	1,250	2005	\$21.0	\$1,772	\$210	\$260	\$448
37	Mesa Colored Water Project	Color	2,500	625	2005	\$10.5	\$886	\$105	\$130	\$448
38	Sweetwater Desalter No.2	TDS	4,000	0	2005	\$6.6	\$964	\$1,070		\$508
POSSIBLE PROJECTS - Subtotal			114,150	29,100		\$313	\$25,757	\$34,081	Ave =	\$589
GROUNDWATER RECOVERY PROGRAM TOTAL			191,425	58,779						

**SOUTHERN CALIFORNIA'S
INTEGRATED WATER RESOURCES PLAN**

**APPENDIX C:
GROUNDWATER CONJUNCTIVE USE
STORAGE POTENTIAL**

Report No. 1107

January, 1996

APPENDIX C:

GROUNDWATER CONJUNCTIVE USE STORAGE POTENTIAL

This appendix summarizes the groundwater basin storage assumptions used in the IRP resource simulation. Most of the data was provided by consultants working for the Association of Groundwater Agencies (AGWA). Other data was based on water master reports and annual water surveys of the groundwater agencies and Member Agencies, collected by Metropolitan. The following presents a brief description of the terms used in this report.

Conjunctive Use Storing:

Storing excess imported water in the local groundwater basins for regional purposes. The stored water could be used for drought protection and/or to reduce seasonal peaks on Metropolitan.

Storage Capacity:

The total volume (or space) of the groundwater basin dedicated to conjunctive use (storing excess imported water for regional benefits). It does not represent the total capacity of the basin, which can be significantly greater. It also does not represent the actual monthly or annual groundwater production, which is usually much less.

Maximum Production Capacity:

The maximum pumping (well) capacity in the basin, which can be expressed in monthly or annual amounts. It represents the maximum quantity of water that could be pumped from the basin in a given time period.

Typical Groundwater Production:

The typical (average) amount of water that is pumped from the basin to meet demand (usually expressed as monthly or annual amounts). Its monthly pattern usually follows the pattern of water demand, because groundwater usually represents the cheapest supply available to the local agency.

Conjunctive Use Production Capacity:

The additional production capacity available for conjunctive use storage. It represents the difference between the maximum production (pumping) capacity and the typical groundwater production for a given month.

Spreading/Injection Capacity:

The physical spreading and/or injection capacity in the groundwater basin available for putting (storing) water. Spreading facilities are usually percolation ponds, while injection facilities are usually large injection pumps.

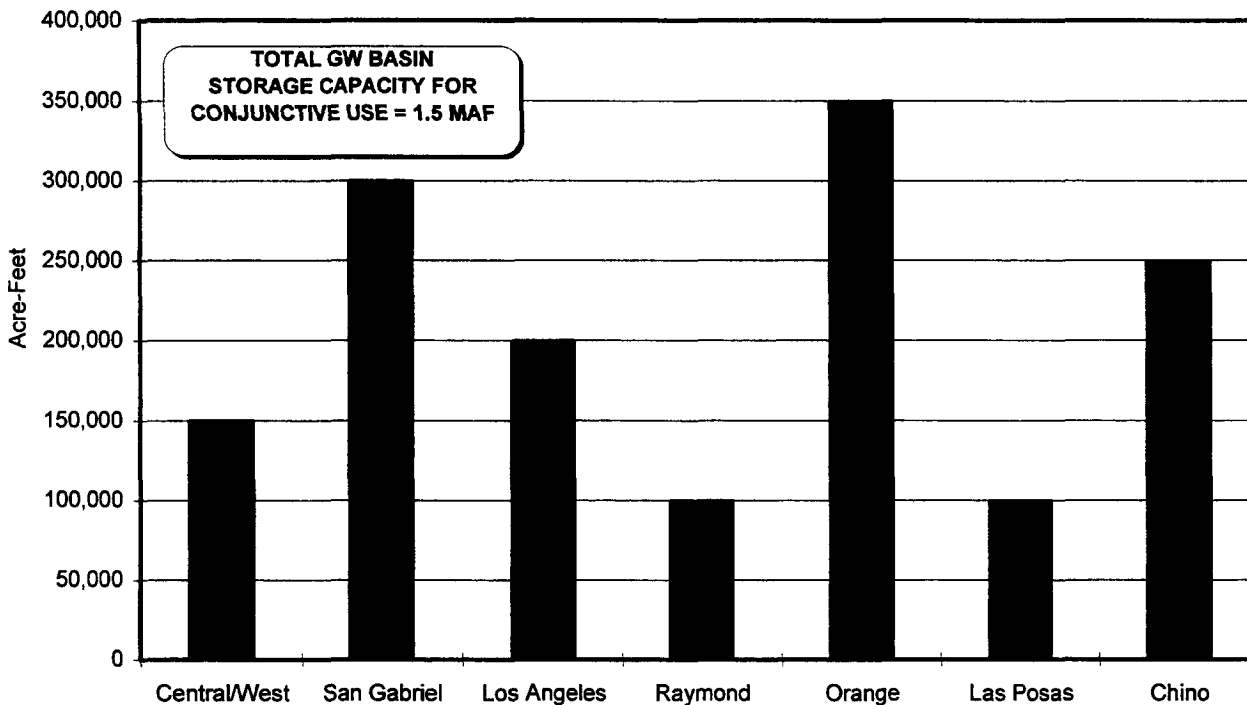
In-Lieu Capacity:

The amount of imported water that local agencies can receive in-lieu of water being pumped from the basin. This has the effect of storing water in the basin for later use. The capacity for in-lieu is limited by: (1) the ability of the individual groundwater agency to take direct deliveries of imported water; (2) the local agencies' water demand; and (3) Metropolitan's conveyance distribution system.

For the purposes of the IRP simulation, monthly values for groundwater production, spreading, and in-lieu capacities were used. It should also be noted that all of the groundwater values presented in this report are the usable amounts available for Metropolitan’s service area only. For example, Chino and Raymond Basins serve areas outside of Metropolitan’s region.

Figure C-1 presents the total storage capacity made available for conjunctive use for each of the major basins. In total, about 1.5 million acre-feet of groundwater storage could be used by the region for emergency, drought, and seasonal purposes. This storage capacity does not represent the amount of additional groundwater production that could be used in any given year -- that amount is significantly less. Of the major basins, Orange County has the greatest potential for storage capacity at 350,000 acre-feet. San Gabriel and Chino Basins also have significant storage potentials, estimated to be 300,000 acre-feet and 250,000 acre-feet, respectively. Raymond and Las Posas both have about 100,000 acre-feet of storage potential. These storage capacities were provided by AGWA’s consultants.

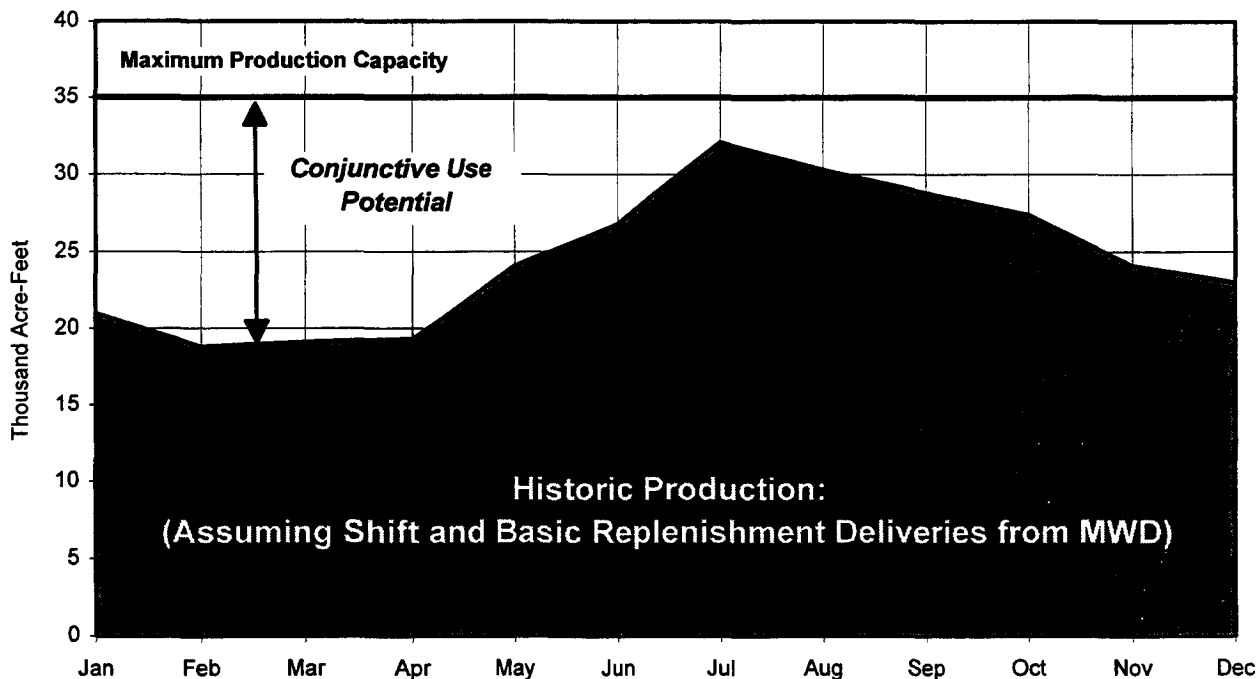
Figure C-1
Groundwater Storage Capacity Available for Conjunctive Use Storage



In order to develop the monthly production capacity available for conjunctive use, two pieces of data are needed: (1) the maximum monthly production (well) capacity; and (2) the historic (typical) monthly groundwater production pattern. Figure C-2 presents an example of this calculation for a specific groundwater basin. The maximum monthly production for this basin is 35,000 acre-feet (represented by the dark line running across the graph). The basin’s historic monthly production pattern is represented by the dark shaded area. In any given month, the difference between the maximum monthly pumping capacity and the historic monthly production equals the remaining pumping capacity available for conjunctive use. For example, in the month of March about 20,000 acre-feet is typically produced from the basin, while the maximum monthly production capacity is

35,000 acre-feet. The difference between the two values, estimated to be about 15,000 acre-feet, is the additional production that could be used for regional storage purposes. During the summer months, the additional production capacity for conjunctive use storage is significantly less.

Figure C-2
Estimating the Potential for Groundwater Storage



The maximum monthly production (well) capacities for each of the major basins were provided by AGWA's consultants. They basically represent existing facilities, except for Orange, Chino, Raymond and Las Posas Basins -- where additional facilities were assumed. The historic monthly production estimates were based on 1985-1989 safe-yield production data obtained by Metropolitan through its annual surveys. These historic monthly production estimates were reviewed by AGWA and the Member Agencies. Figure C-3 presents the average winter and summer month production capacity potential for conjunctive use storage by basin. In general, the largest potential for conjunctive use storage is during the winter, when water demands in the basin are low. However, in most cases the need for significant conjunctive use storage production is during the summer.

In order to estimate how much water could be stored in the basins, two pieces of data are required: (1) the maximum monthly spreading capacity; and (2) estimates of monthly natural runoff. The difference between the two values indicates the remaining spreading capacity for storing excess imported water for regional purposes. Maximum monthly spreading capacities for each basin were provided by AGWA's consultants. Estimates of natural runoff were calculated from data provided by flood control districts and/or by the groundwater agency reports. Figure C-4 presents an example of the spreading capacity for a basin.

Figure C-3
Monthly Groundwater Production

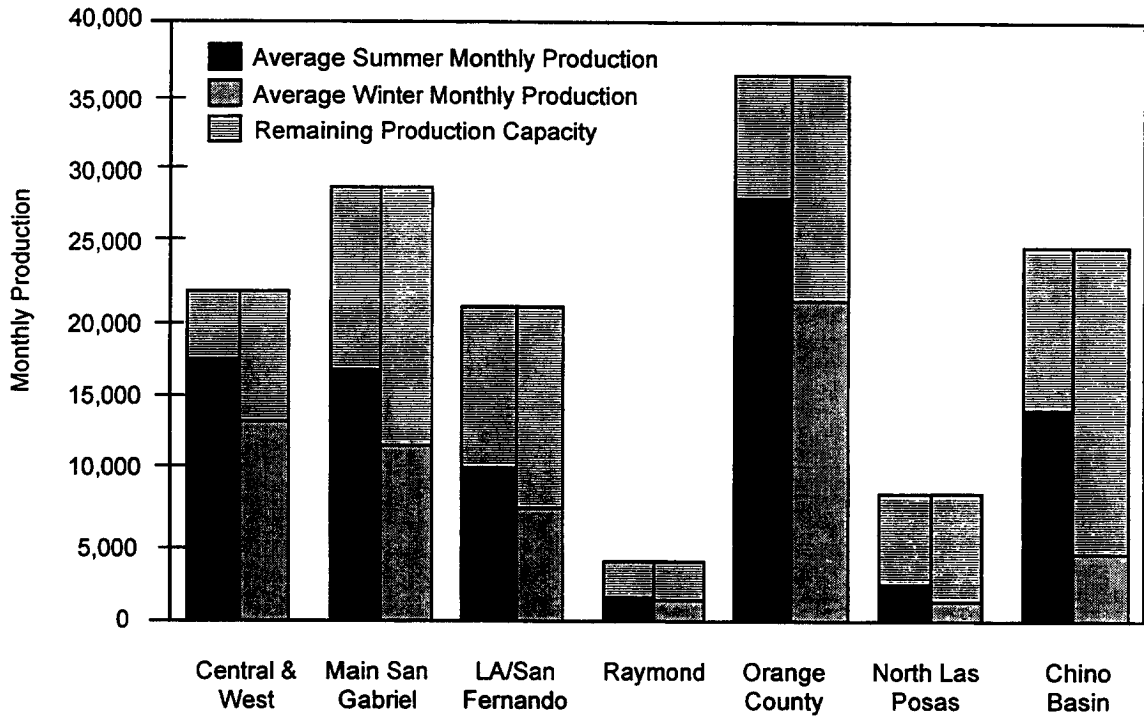
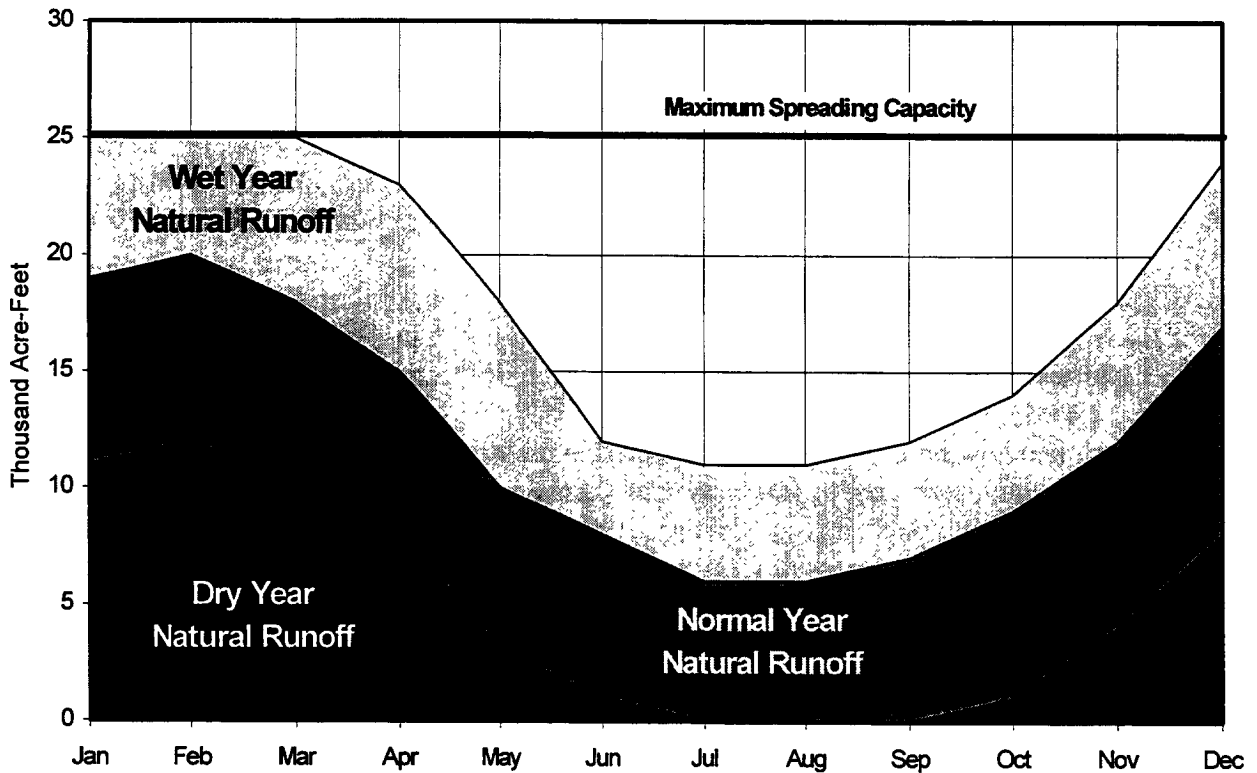
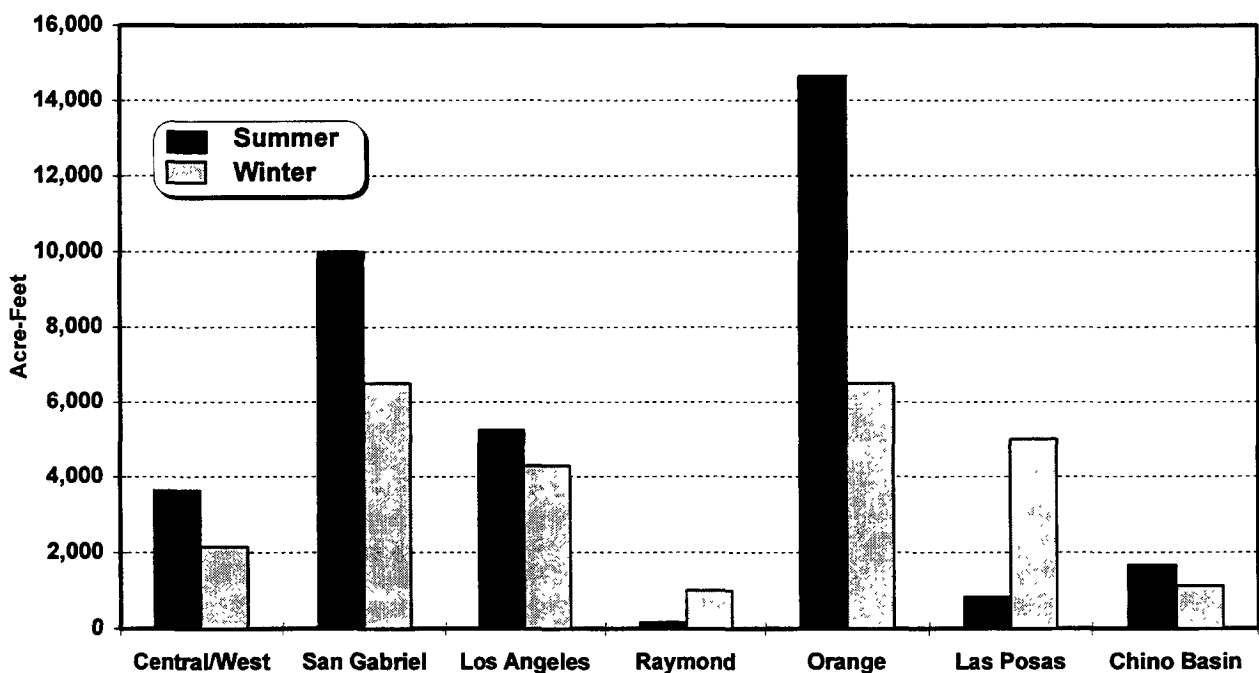


Figure C-4
Groundwater Basin Spreading Capacity



As shown in Figure C-4, winter months have lower spreading capacities for storing excess imported water because the basin is making use of natural runoff. This calculation gets somewhat complicated because in addition to winter vs. summer runoff data, the type of local hydrologic year must also be taken into account. For example, during local wet years natural runoff is very high -- even during the summer. In fact, for most basins wet year runoff prevents any winter-time spreading of imported water. However, it is important to note that the majority of excess imported water is available during winter months and these local wet and normal years (because northern California hydrology typically mirrors local hydrology). A benefit of the Eastside Reservoir Project is that excess imported water can be stored in the surface reservoir during the winter and then cycled into the groundwater basins during the summer months -- when groundwater spreading capacities are the greatest. Figure C-5 presents the winter and summer month spreading/injection capacities for each basin available for additional conjunctive use storage.

**Figure C-5
Monthly Spreading/Injection Capacities for Conjunctive Use Storage**



Another way to store excess water into the groundwater basins is by in-lieu deliveries of Metropolitan water. This method does not require spreading facilities or connections to physically get water into the basin. Instead of pumping from the groundwater basin, direct deliveries of imported water are made to the local groundwater pumping agency. These deliveries are made in-lieu of the agency pumping groundwater.

For example: Member Agency X usually pumps an average of 30,000 acre-feet per month from the basin during the winter and buys no Metropolitan non-interruptible water. When excess imported water is available -- Metropolitan makes available discount water to be sold in-lieu of Member Agency X pumping the water from the basin. The Member Agency still meets its demand and keeps the groundwater supply it would have pumped for later use.

The limitations to in-lieu deliveries as a means to store water include: (1) local ground-water pumping agencies that cannot receive imported water (either directly from Metropolitan or indirectly through local interconnections) cannot take advantage of the excess imported water; and (2) Metropolitan's distribution system is pushed harder because instead of delivering its typical non-interruptible water, more water is being delivered to for in-lieu purposes. Table C-1 presents a summary of the storage parameters used in the resource simulation model regarding groundwater storage.

**Table C-1
Groundwater Storage Parameters**

Storage Parameter	Time Period	Central/ West	San Gabriel	LA/San Fernando	Ray- mond	Orange	Las Posas	Chino
Storage Capacity for Conjunctive Use (acre-feet)		150,000	300,000	200,000	100,000	350,000	100,000	250,000
Available Monthly Production Capacity (acre-feet)*		22,000	29,000	21,000	4,000	36,500	8,500	25,000
In-lieu Capacity for Conjunctive Use, expressed as percent of monthly groundwater safe-yield production **	1996	40%	25%	55%	80%	40%	3%	30%
	2000	40%	30%	60%	85%	45%	3%	45%
	2010	50%	30%	70%	85%	60%	3%	45%
Wet Year Spreading of Additional Imported Water (acre-feet)	Jan	0	0	0	1,000	0	5,000	0
	Feb	0	0	0	1,000	0	5,000	0
	Mar	0	0	0	1,000	0	5,000	0
	Apr	0	0	0	1,000	0	5,000	1,000
	May	1,000	0	2,500	0	0	0	1,800
	Jun	2,200	7,000	2,700	0	12,000	0	1,800
	Jul	2,500	10,000	3,500	0	14,000	0	2,000
	Aug	3,000	11,000	4,000	0	15,000	0	1,800
	Sep	2,500	10,000	4,000	0	15,000	0	1,000
	Oct	2,200	8,000	2,200	1,000	14,000	5,000	1,000
	Nov	1,000	5,000	1,000	1,000	8,000	5,000	0
	Dec	0	0	0	1,000	0	5,000	0
Normal Year Spreading of Additional Imported Water (acre-feet)	Jan	1,500	4,000	3,000	1,000	0	5,000	500
	Feb	2,000	5,000	4,600	1,000	5,000	5,000	1,200
	Mar	2,400	8,000	5,200	1,000	6,500	5,000	1,500
	Apr	2,500	9,000	5,400	1,000	6,500	5,000	2,000
	May	3,500	10,000	5,400	0	13,000	0	2,000
	Jun	3,800	10,000	5,400	0	15,000	0	2,000
	Jul	4,000	11,000	5,400	0	15,000	0	2,000
	Aug	4,000	11,000	5,400	0	15,000	0	2,000
	Sep	3,500	10,000	5,100	0	15,000	0	1,000
	Oct	3,000	8,000	4,700	1,000	15,000	5,000	1,000
	Nov	2,500	8,000	4,500	1,000	13,000	5,000	1,000
	Dec	2,000	5,000	3,000	1,000	8,000	5,000	500
Dry Year Spreading of Additional Imported Water (acre-feet)	Jan	3,000	20,000	5,600	1,000	20,000	5,000	1,800
	Feb	3,300	21,000	5,700	1,000	21,000	5,000	2,000
	Mar	3,500	25,000	6,500	1,000	25,000	5,000	2,200
	Apr	4,000	28,000	6,700	1,000	28,000	5,000	2,500
	May	4,300	30,000	6,700	0	30,000	0	2,700
	Jun	4,300	30,000	6,700	0	30,000	0	2,700
	Jul	4,300	30,000	6,700	0	30,000	0	2,700
	Aug	4,300	30,000	6,700	0	30,000	0	2,700
	Sep	4,000	28,000	6,400	0	28,000	0	2,700
	Oct	3,500	25,000	5,900	1,000	25,000	5,000	2,500
	Nov	3,300	21,000	5,600	1,000	21,000	5,000	2,200
	Dec	3,000	20,000	5,700	1,000	20,000	5,000	1,900

* Additional monthly capacity available for conjunctive use represents the difference between this maximum production capacity and the typical monthly groundwater production.

** Represents only the in-lieu deliveries for conjunctive use purposes; in-lieu potential improves over time as improvements are made to MWD's distribution system.

Table C-2 presents the typical (average of 1985-1989) groundwater safe-yield production and additional production from conjunctive use storage for the major basins in Metropolitan's service area. Note that Santa Monica, Eastern, and Western groundwater basins are shown in Table C-2, but not in Table C-1. This is because the storage potential in these basins are not significant and/or could not be determined at this time. However, these basins do provide year-round local supplies to the region and are therefore included in the analysis.

**Table C-2
Average Groundwater Production**

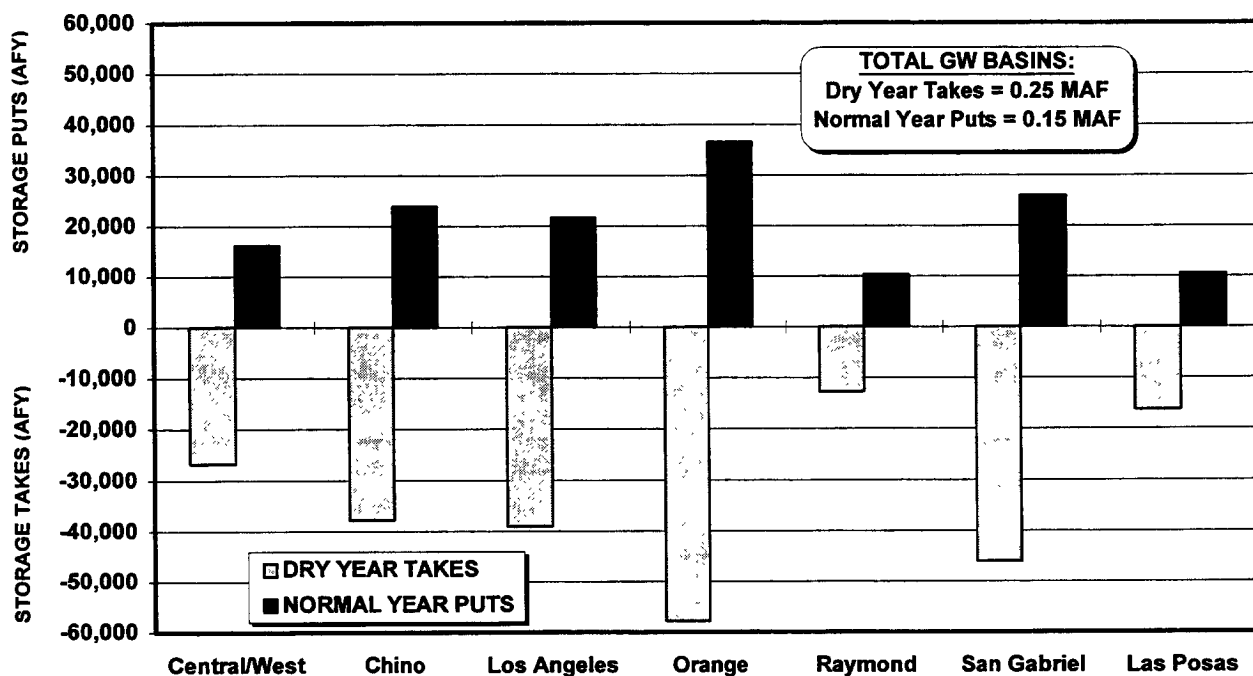
Historic Groundwater Safe-Yield Production From 1980-1989 *											
	Central/ West	San Gabriel	LA/San Fernando	Raymond	Santa Monica	Orange	Las Posas	Chino	Eastern	Western	Total
Jan	13,301	11,101	7,577	1,377	451	22,008	1,156	7,185	2,253	5,611	72,019
Feb	12,192	10,589	6,723	1,245	407	19,034	1,063	6,546	2,170	4,790	64,759
Mar	13,116	11,784	7,150	1,226	363	19,034	1,202	7,824	2,754	5,748	70,200
Apr	14,040	13,150	8,110	1,415	385	19,629	1,688	10,857	5,258	8,758	83,290
May	16,072	15,883	9,604	1,472	402	24,090	2,289	15,647	8,597	13,548	107,602
Jun	17,180	17,420	10,458	1,321	418	26,766	2,659	18,681	11,852	16,559	123,312
Jul	19,212	18,445	11,098	2,056	550	32,120	2,821	21,555	14,188	19,296	141,340
Aug	18,843	17,932	11,098	2,019	556	30,335	2,705	20,597	12,352	18,611	135,047
Sep	17,180	16,054	10,031	1,811	495	28,848	2,474	17,244	9,848	15,601	119,585
Oct	16,072	14,517	9,070	1,811	506	27,361	2,219	14,849	7,428	13,274	107,107
Nov	14,224	12,467	8,110	1,660	473	24,090	1,526	10,378	4,173	8,621	85,723
Dec	13,301	11,443	7,683	1,453	495	24,090	1,318	8,303	2,587	6,432	77,103
Total	184,731	170,785	106,712	18,865	5,500	297,404	23,119	159,663	83,462	136,848	1,187,088
Additional Groundwater Production for Conjunctive Use Storage **											
	Central/ West	San Gabriel	LA/San Fernando	Raymond	Santa Monica	Orange	Las Posas	Chino	Eastern	Western	Total
Jan	8,699	17,899	13,423	2,623	NA	14,492	7,344	17,815	NA	NA	82,296
Feb	9,808	18,411	14,277	2,755	NA	17,466	7,437	18,454	NA	NA	88,608
Mar	8,884	17,216	13,850	2,774	NA	17,466	7,298	17,176	NA	NA	84,664
Apr	7,960	15,850	12,890	2,585	NA	16,871	6,812	14,143	NA	NA	77,112
May	5,928	13,117	11,396	2,529	NA	12,410	6,211	9,353	NA	NA	60,944
Jun	4,820	11,580	10,542	2,679	NA	9,734	5,841	6,319	NA	NA	51,516
Jul	2,788	10,555	9,902	1,944	NA	4,380	5,679	3,445	NA	NA	38,694
Aug	3,157	11,068	9,902	1,981	NA	6,165	5,795	4,403	NA	NA	42,472
Sep	4,820	12,946	10,969	2,189	NA	7,652	6,026	7,756	NA	NA	52,359
Oct	5,928	14,483	11,930	2,189	NA	9,139	6,281	10,151	NA	NA	60,101
Nov	7,776	16,533	12,890	2,340	NA	12,410	6,974	14,622	NA	NA	73,545
Dec	8,699	17,557	13,317	2,547	NA	12,410	7,182	16,698	NA	NA	78,411
Total	79,269	177,215	145,288	29,135	NA	140,596	78,881	140,337	NA	NA	790,721
Winter	51,827	103,466	80,648	15,624	NA	91,116	43,047	98,908	NA	NA	484,635
Summer	27,442	73,749	64,641	13,511	NA	49,480	35,834	41,429	NA	NA	306,086

* Does not include Metropolitan's basic replenishment, which averages to be about 100,000 acre-feet per year for all basins.

** Calculated by subtracting the historic monthly safe-yield production from the maximum monthly production capacity in Table C-1.

Based on the results of the resource simulation model, the following dry year storage production (takes from storage) and normal year spreading, injection, and in-lieu deliveries (puts into storage) were estimated for each basin. Dry years are estimated to occur 1 in 10 years, and normal years are estimated to occur 7 in 10 years. Figure C-6 presents this storage summary. In total, the average (from 1995 to 2020) additional groundwater production (takes from storage) is about 250,000 acre-feet per year. In some years this storage production is much greater -- about 350,000 acre-feet, while in other years it is much less -- about 100,000 acre-feet. The variation has to do with the projection of demands, core local supplies, and available imported supplies. In total, the average (from 1995 to 2020) spreading and in-lieu deliveries (puts into storage) is about 150,000 acre-feet per year. Orange County has the greatest potential for storage, followed by San Gabriel, Chino, and Los Angeles.

Figure C-6
Storage Simulation Results Indicating the Average Storage Puts and Takes



**SOUTHERN CALIFORNIA'S
INTEGRATED WATER RESOURCES PLAN**

**APPENDIX D:
STATE WATER PROJECT SUPPLY VARIATION
AND DEVELOPMENT POTENTIAL**

Report No. 1107

January, 1996

APPENDIX D:

STATE WATER PROJECT SUPPLIES AND MODELING

For the IRP, Metropolitan needed to capture the effect of two potential variations in SWP supplies. First, the effect of hydrologic conditions on SWP supplies needed to be determined. Second, the effect of different levels of investment on SWP operational standards needed to be determined. To answer each of these questions, Metropolitan started with projected SWP supplies that were generated by the California Department of Water Resources (DWR) simulation model, DWRSIM.

DWRSIM is used by DWR to forecast SWP water supplies for the 29 State Water Contractors (Contractors). As inputs, DWRSIM uses a set of operational constraints or “standards” for water operations in the Delta, a level of investment and development on the SWP, and a demand for water by the Contractors. For a given set of operational rules, level of investment, and water demand, DWRSIM cycles through historical hydrologic conditions and calculates the supply yield that would result from those conditions. The supply yield is calculated for each historical hydrologic year used by DWR, from 1922 through 1991, and includes the carryover storage effect along the SWP system.

For Metropolitan’s IRP modeling, four levels of SWP investment were requested from DWRSIM. In each of the four DWRSIM runs, a full project demand of 4.23 million acre-feet was requested, corresponding to a 2.01 million acre-foot request by Metropolitan. Metropolitan made this assumption because it was necessary to know the potential amount of water supply available, with all Contractors requesting their full allocation. Operational constraints on the SWP were specified using the State Water Resources Control Board proposed Decision 1630 (D-1630). Although D-1630 had not been adopted, the standards were considered to be a reasonable surrogate for anticipated operational constraints in the Delta. The four investment levels represented the different development paths that could occur on the SWP. By requesting four sets of DWRSIM output based on four development paths, Metropolitan could impose completion of the development levels at different points in the planning horizon. The four levels of investment specified for IRP modeling are: (1) Existing Facilities, (2) Interim Delta Improvements, (3) Full Delta Fix, and (4) South of the Delta Storage.

Under the “existing facilities” scenario, no new investment is made on the SWP. This scenario most closely represents current conditions on the SWP and in the Delta. For the IRP modeling, a degradation path was assumed with the “existing facilities” supply scenario. The current political and environmental controversy surrounding water supply issues in the Delta led to the assumption that, without any improvements on the SWP, potential water supply would decrease over time. It was specifically assumed that in each future year, the amount of water that was available under D-1630 would degrade 5% incrementally until the year 2005. With degradation, supplies available under the “existing facilities” scenario would equal one-half of the current supplies available under D-1630 operational constraints by the year 2005.

Under the “Interim Delta Improvements” scenario, investments that improve the conditions at the South end of Delta are assumed to occur. In the IRP modeling, “Interim Delta Improvements” are assumed to occur in the year 2000, providing an increase in expected supply yield. However, because the improvements are understood to be “interim” and provide only a temporary “fix” to Delta problems, the available supply is degraded over time. The degradation path occurs over a ten year period. The supply available under the “Interim South Delta Improvements” scenario would degrade gradually until it became equal to 75% of the current supplies available under the “existing facilities” scenario.

Under the “Full Delta Fix” scenario, a “fix” to the Delta, presumably in the form of a peripheral canal, results in a significant increase in the amount and reliability of SWP supply. In the IRP modeling, the “Full Delta Fix” is assumed to be on-line in 2010. Since the “Full Delta Fix” involves a permanent fix to many issues surrounding Delta water exports, no degradation is assumed when using this scenario. Supply varies only by hydrology.

Under the “South of the Delta Storage” scenario, nearly 3 million acre-feet of storage capacity is added to the SWP south of the Delta. In conjunction with the implementation of the “Full Delta Fix” facilities, this scenario provides a full SWP allocation of 2 million acre-feet nearly 85% of the time. This facility is assumed to be available by the year 2015, and because the scenario is created by permanent facilities, no degradation path is assumed.

For IRP modeling purposes, the four scenarios could be joined together at different points in the planning horizon to form the assumption of a specific development path on the SWP. In the Preferred Resources Mix SWP assumption, the “existing facilities” case was used for forecast years 1995-1999. The “Interim Delta Improvements” case was brought on line in the forecast year 2000 and was effective until the year 2009. In 2010, the “Full Delta Fix” was implemented and assumed to be the scenario describing SWP deliveries through 2020, the end of the planning horizon.

Table D-1 shows the matrix of available SWP for existing facilities under operational rule D-1630. The forecast years are shown across the top of the table and the hydrologic trace years are shown along the side of the table. Tables D-2 through D-4 show similar data for the Interim Improvements, Full Delta Fix, and South of Delta Storage, respectively.

If the data in Tables D-1 through D-4 were ranked by percentile and joined together into development paths, as described above, then the available SWP supplies during certain types of hydrologic years could be estimated. For example, what would the top 10 percentile projected SWP supply be? Figures D-1 through D-3 show the projected SWP supplies and development potential under the top 10 percentile (hot and dry conditions), the middle 50 percentile (normal hydrology), and the bottom 90 percentile (cool and wet conditions).

Figure D-1
Projected SWP Supplies Assuming Top 10 Percentile of Hydrology

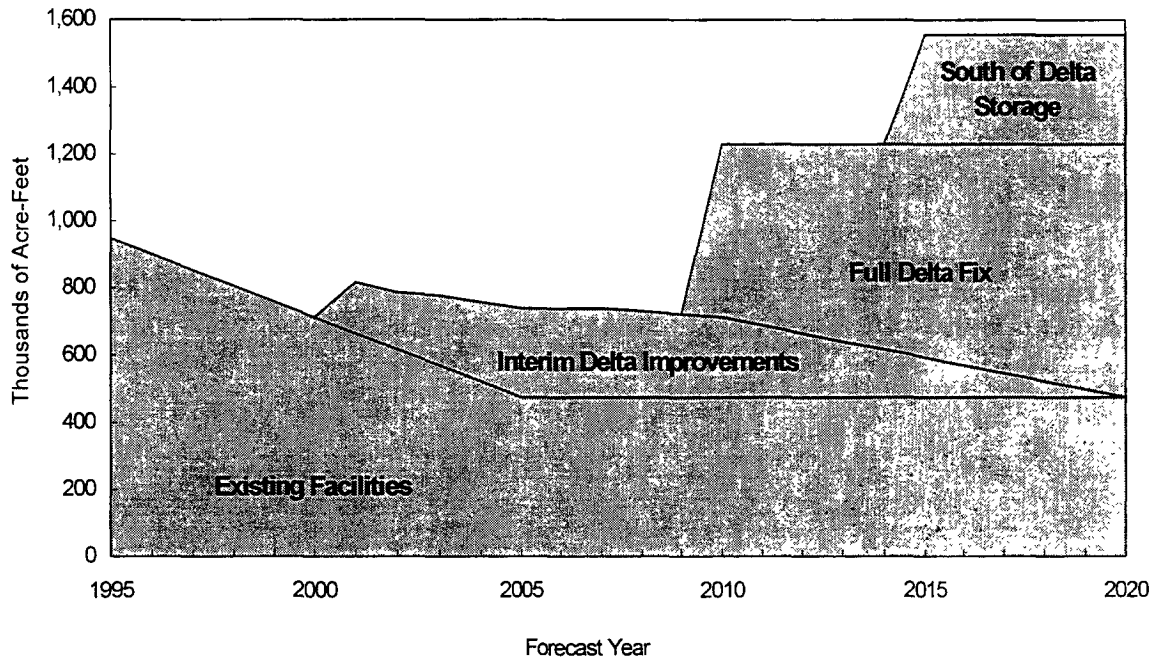


Figure D-2
Projected SWP Supplies Assuming Middle 50 Percentile of Hydrology

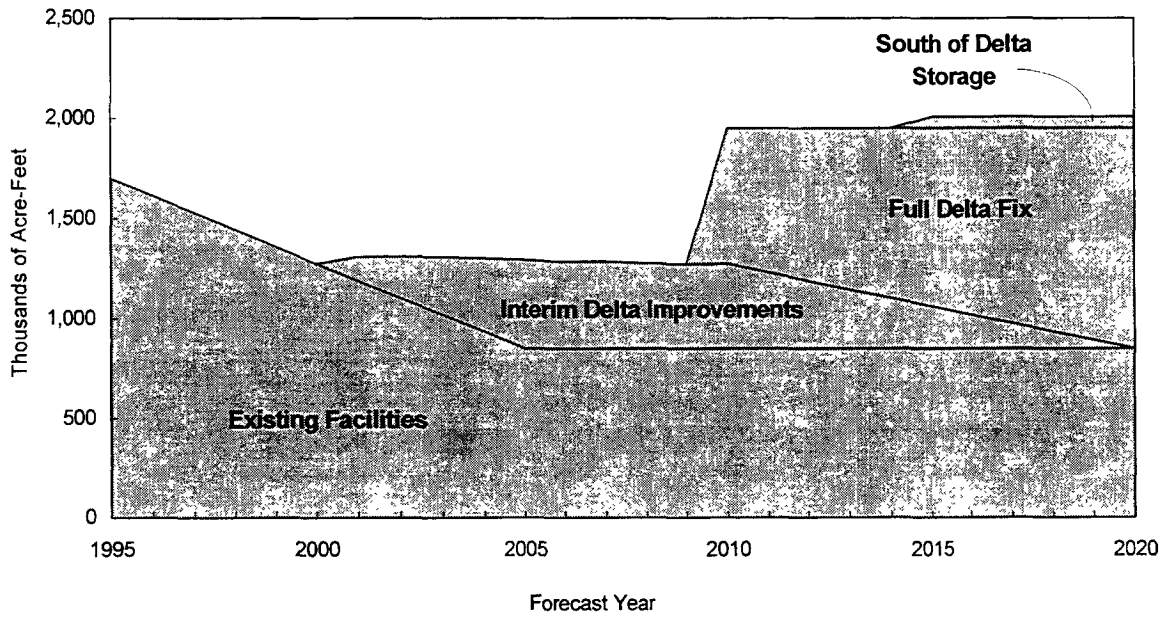
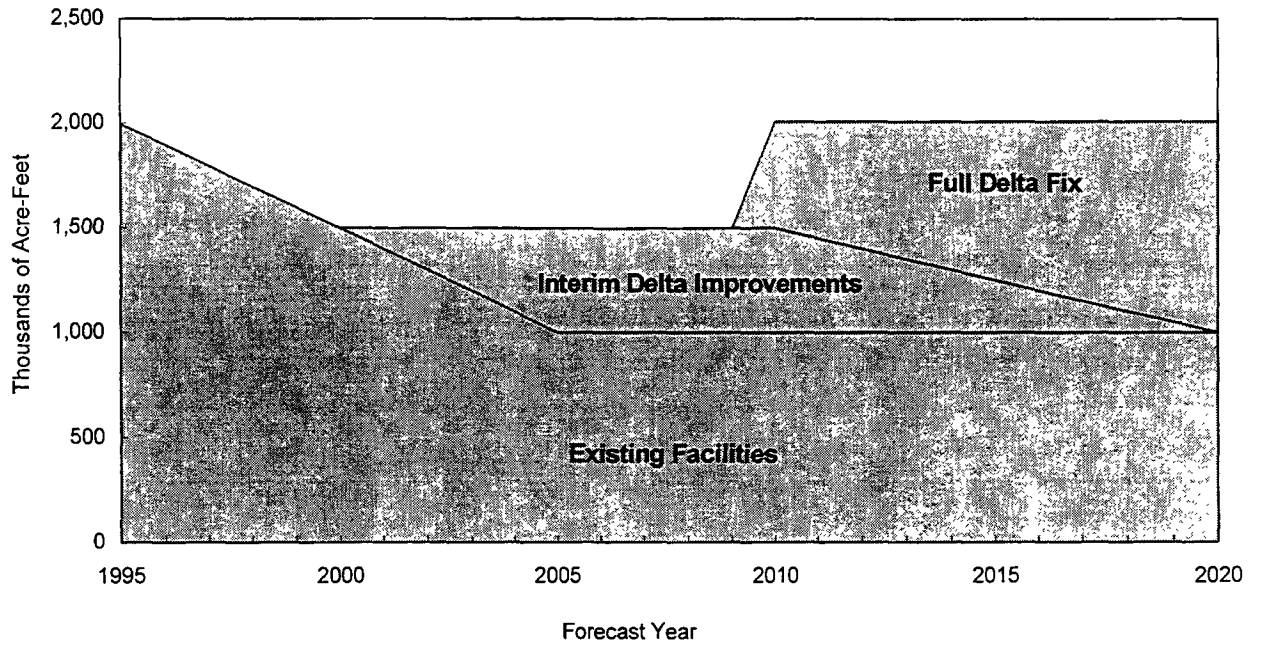


Figure D-3
Projected SWP Supplies Assuming Bottom 90 Percentile of Hydrology



**SOUTHERN CALIFORNIA'S
INTEGRATED WATER RESOURCES PLAN**

**APPENDIX E:
MWD CAPITAL PROJECTS**

Report No. 1107

January, 1996

APPENDIX E MWD CAPITAL PROJECTS

Metropolitan's anticipated capital expenditures have been divided into two broad categories of projects to facilitate financial analyses. The first category, supply, distribution, and storage projects, includes raw water supply and treated water distribution lines, groundwater and surface water storage projects, and projects that maintain the operational reliability and efficiency of Metropolitan's existing conveyance and distribution system. The second category, water treatment projects, includes new water treatment projects to enable Metropolitan to meet existing and future water quality regulations, and upgrades, modifications, or rehabilitation projects at existing treatment facilities so these plants can continue to meet water quality regulations.

The following table summarizes estimated capital costs over 10 years, over 25 years, and shows the total program estimate (including contingencies and actual costs since project inception) for the major projects anticipated. The table reflects the first quarterly update of Metropolitan's capital improvement program. Volume 2 of the final IRP report will be revised to reflect the data contained in this appendix. Costs are escalated at five percent per year as required to reflect the appropriate fiscal year cost. Metropolitan uses the 10-year and 25-year escalated costs in determining revenue requirements and the impact the capital expenditures would have on commodity rates and indebtedness.

The supply, distribution, and storage projects category represents about 80 percent of the 10-year escalated capital costs and 76 percent of the 25-year escalated capital costs. Major projects under this category include the Eastside Reservoir Project, several groundwater conjunctive use projects, the Inland Feeder, San Diego Pipeline No. 6, the CPA Tunnel and Pipeline, the Allen-McColloch Pipeline and the South County Pipeline. Other major projects include repair or replacement of the outlet tower at Lake Mathews, a supervisory control and data acquisition system for the CRA, seismic upgrades along the CRA, the Union Station long-term headquarters and the Desalination Demonstration Project.

The water treatment projects category accounts for the remaining 20 percent of capital expenditures for the next 10 years and 24 percent of the remaining capital expenditures over the next 25 years. New major water treatment projects include the CPA Filtration Plant, the Perris Filtration Plant, the oxidation retrofit program for the five existing filtration plants, completing expansions of the Mills and Jensen filtration plants, and a second finished water reservoir at Diemer. Other major projects include the Cryptosporidium action plan, and various modifications or upgrades at the five existing filtration plants to enable these plants to continue to meet water quality regulations.

Program No	Title	1995-96	1996-97	1997-98	1998-99	1999-00	2000-01	2001-02	2002-03	2003-04	2004-05	10 Yr. CIP 96/96-04/05 Total
WATER TREATMENT PROJECTS												
5-6270-61	All Facilities - Discharge Elimination	357.0	1,730.4	75.4	-	-	-	-	-	-	-	2,162.9
5-6030-31	All Filtration Plants and Distribution System - Chemical Spill Containment	9,394.4	894.7	-	-	-	-	-	-	-	-	10,289.1
5-0122-33	Diemer - Relocate Front Entrance Gate and Install Lighting, Sec Camera and Gate Control	-	-	239.4	539.7	-	-	-	-	-	-	779.0
5-6810-31	Diemer and Weymouth Install Emergency Generators	1,580.2	1,724.4	-	-	-	-	-	-	-	-	3,304.6
5-6820-31	Diemer Chemical Feed Pumps Relocation	558.4	335.6	-	-	-	-	-	-	-	-	894.0
5-6760-31	Diemer Filtration Plant - Chlorination System Modifications	1,424.9	1,109.4	-	-	-	-	-	-	-	-	2,534.2
5-0503-31	Diemer Filtration Plant - Construct Sedimentation Basin Spillway	827.7	2,004.1	-	-	-	-	-	-	-	-	2,831.8
5-0509-31	Diemer Filtration Plant - Domestic Water System Improvement	795.8	44.4	-	-	-	-	-	-	-	-	840.2
5-6570-31	Diemer Filtration Plant - Mong and Settling Basin No. 8 - North Slope Remediation	1,105.5	251.0	-	-	-	-	-	-	-	-	1,356.5
5-0501-31	Diemer Filtration Plant - New Finished Water Reservoir	667.4	2,621.9	20,377.6	14,410.1	15,261.3	326.5	-	-	-	-	63,664.7
5-0502-61	Diemer Filtration Plant - New Maintenance Building	-	2,910.9	926.2	-	-	-	-	-	-	-	3,837.1
5-6990-11	Diemer Filtration Plant - Upgrade Flocculator Drives	330.3	-	-	-	-	-	-	-	-	-	330.3
5-5500-31	Diemer Filtration Plant Modifications and Washwater Reclamation Plant Enlargement	1,869.3	-	-	-	-	-	-	-	-	-	1,869.3
5-6640-31	Diemer Land Acquisition and Habitat Conservation Plan	2,721.9	2,668.9	-	-	-	-	-	-	-	-	5,390.8
5-0520-31	Diemer, Weymouth & Skinner Filtrations Plants - Oxidation Retrofit Program	-	8,553.9	36,275.6	22,361.1	51,557.4	106,356.5	72,392.9	9,288.1	-	-	306,785.4
5-6080-32	Filt.Pits., Distr. System, and Colorado River Aqueduct - Backflow Prevention Assemblies	2,837.1	3,518.8	1,679.3	440.8	462.9	484.7	508.9	533.1	559.7	587.5	11,612.8
5-6100-31	Jensen & Mills Filtration Plants - Oxidation Retrofit Program	20,002.1	58,259.3	64,143.2	23,064.6	-	-	-	-	-	-	165,469.3
5-6270-31	Jensen Filtration Plant - Expansion No. 1	10,442.0	4,496.3	469.0	-	-	-	-	-	-	-	15,407.3
5-0508-31	Jensen Filtration Plant - Repair Roof at Reservoir No 1	810.6	602.0	-	-	-	-	-	-	-	-	1,412.7
5-5820-32	Jensen Filtration Plant - Replace Filter Media	-	-	-	-	-	-	-	-	-	-	-
5-6980-32	Jensen Filtration Plant - Sludge Handling Study	302.8	-	-	-	-	-	-	-	-	-	302.8
5-6860-31	Jensen Plant - Chemical Tank Farm Modifications	147.9	-	-	-	-	-	-	-	-	-	147.9
5-0112-63	La Verne Facilities - Construct a Utility Shop Building	-	-	532.3	549.2	7,223.4	-	-	-	-	-	8,305.0
5-0317-61	La Verne Facilities - Electrical Service Upgrade	-	575.8	-	-	-	-	-	-	-	-	575.8
5-6550-61	La Verne Facility - Hazardous Waste Staging Area	5.8	2,011.3	-	-	-	-	-	-	-	-	2,017.1
5-5570-31	Mills Filtration Plant - Expansion No 2	28,061.6	17,017.4	1,088.0	-	-	-	-	-	-	-	46,167.0
5-0111-31	Mills Filtration Plant - Landfill	-	390.4	2,241.1	1,139.1	1,850.8	1,968.4	-	-	-	-	7,589.8
5-5610-21	San Joaquin Reservoir - Improvement	79.4	595.4	13,564.8	7,076.5	-	-	-	-	-	-	21,316.0
5-7010-11	San Joaquin Reservoir - Slope Repair (Met's Share)	244.4	249.7	-	-	-	-	-	-	-	-	494.1
5-6280-33	Skinner - Relocate Front Entrance Gate and Fencing, and Construct New Parking Lot	-	-	239.8	-	-	-	-	-	-	-	239.8
5-6110-31	Skinner Filtration Plant - Emergency Power Generating System	1,721.0	94.7	-	-	-	-	-	-	-	-	1,815.6
5-6660-31	Skinner Filtration Plant - Filter Media Replacement	1,185.7	4,110.1	31.4	-	-	-	-	-	-	-	5,327.2
5-0515-31	Skinner Filtration Plant - Flocculator Replacement in Modules 1 & 2	177.6	1,326.7	-	-	-	-	-	-	-	-	1,504.2
5-0304-31	Skinner Filtration Plant - Install Effluent Adjustable Wer	-	549.0	142.7	-	-	-	-	-	-	-	691.7
5-0410-31	Skinner Filtration Plant - Modules 4, 5, and 6 Sedimentation Basin	-	1,971.4	11,176.0	22,168.0	3,881.1	-	-	-	-	-	39,198.4
5-6510-31	Skinner Filtration Plant Mono#	3.1	724.5	480.5	-	-	-	-	-	-	-	1,208.0
5-6920-31	Skinner Modules 1-3 Electrical Conduit and Wireways Replace	221.8	221.9	-	-	-	-	-	-	-	-	443.8
5-0402-61	Warehouse and Storage Building At Mills Filtration Plant	11.1	-	1,967.1	357.3	-	-	-	-	-	-	2,335.5
5-0514-31	Water Quality - Cryptosporidium Action Plan	1,364.1	2,409.6	1,259.9	-	-	-	-	-	-	-	5,033.6
5-6590-31	Water Quality - Demonstration-Scale Testing	1,951.5	2,341.1	799.0	-	-	-	-	-	-	-	5,091.6
5-0401-61	Water Quality Lab - Inductively Coupled Plasma Mass Spectrometer	294.4	-	-	-	-	-	-	-	-	-	294.4
5-6350-63	Water Quality Laboratory Building Expansion	2,643.9	5,223.3	4,097.4	-	-	-	-	-	-	-	11,964.6
5-6910-32	Weymouth Filtration Plant - Sludge Handling Facility	65.1	5,109.2	-	-	-	-	-	-	-	-	5,174.3
5-6530-31	Weymouth Filtration Plant - Ferric Chloride Retrofit and Storage Augmentation	357.3	953.9	-	-	-	-	-	-	-	-	1,311.2
5-0002-32	Weymouth Replace Existing Asphalt Paving	-	134.6	773.4	-	-	-	-	-	-	-	908.0
TOTAL WATER QUALITY AND TREATMENT (EXISTING PLANTS)		94,663.2	137,736.9	162,581.1	92,106.3	80,236.9	109,136.0	72,901.8	9,821.2	589.7	587.5	760,229.6
5-5560-71	Central Pool Augmentation and Water Quality Project - Study and Land Acquisition	3,507.2	19,486.9	-	-	-	-	-	-	-	-	22,994.1
5-0221-32	Central Pool Augmentation Filtration Plant	-	-	-	-	-	-	-	-	-	-	-
	Central Pool Augmentation Filtration Plant - 2nd Stage	-	-	-	-	-	-	-	-	-	-	-
TOTAL CENTRAL POOL AUGMENTATION (Filtration Projects)		3,507.2	19,486.9	-	-	-	-	-	-	-	-	22,994.1
5-0516-31	Perris Filtration Plant	-	-	-	-	-	-	-	-	-	-	-
5-5800-71	Perris Filtration Plant - Study and Advance Land Acquisition	-	19,387.4	-	-	-	-	-	-	-	-	19,387.4
TOTAL PERRIS FILTRATION PLANT		-	19,387.4	-	-	-	-	-	-	-	-	19,387.4
SUBTOTAL FOR WATER TREATMENT PROJECTS		98,070.4	176,610.2	162,581.1	92,106.3	80,236.9	109,136.0	72,901.8	9,821.2	589.7	587.5	802,611.1
TOTAL PROPOSED CAPITAL IMPROVEMENT PROGRAM		416,077.6	789,804.7	928,782.0	711,641.3	444,408.5	412,530.5	215,420.8	77,686.5	70,641.2	68,162.7	4,138,155.7

Program No	Title	2005-06	2006-07	2007-08	2008-09	2009-10	2010-11	2011-12	2012-13	2013-14	2014-15
WATER TREATMENT PROJECTS											
5-6270-61	All Facilities - Discharge Elimination	-	-	-	-	-	-	-	-	-	-
5-6030-31	All Filtration Plants and Distribution System - Chemical Spill Containment	-	-	-	-	-	-	-	-	-	-
5-0122-33	Diemer - Relocate Front Entrance Gate and Install Lighting, Sec Camera and Gate Control	-	-	-	-	-	-	-	-	-	-
5-6810-31	Diemer and Weymouth Install Emergency Generators	-	-	-	-	-	-	-	-	-	-
5-6820-31	Diemer Chemical Feed Pumps Relocation	-	-	-	-	-	-	-	-	-	-
5-6760-31	Diemer Filtration Plant - Chlorination System Modifications	-	-	-	-	-	-	-	-	-	-
5-0503-31	Diemer Filtration Plant - Construct Sedimentation Basin Spillway	-	-	-	-	-	-	-	-	-	-
5-0509-31	Diemer Filtration Plant - Domestic Water System Improvement	-	-	-	-	-	-	-	-	-	-
5-6570-31	Diemer Filtration Plant - Mixing and Settling Basin No. 8 - North Slope Remediation	-	-	-	-	-	-	-	-	-	-
5-6501-31	Diemer Filtration Plant - New Finished Water Reservoir	-	-	-	-	-	-	-	-	-	-
5-0502-61	Diemer Filtration Plant - New Maintenance Building	-	-	-	-	-	-	-	-	-	-
5-6890-11	Diemer Filtration Plant - Upgrade Flocculator Drives	-	-	-	-	-	-	-	-	-	-
5-5500-31	Diemer Filtration Plant Modifications and Washwater Reclamation Plant Enlargement	-	-	-	-	-	-	-	-	-	-
5-6640-31	Diemer Land Acquisition and Habitat Conservation Plan	-	-	-	-	-	-	-	-	-	-
5-0520-31	Diemer, Weymouth & Skinner Filtrations Plants - Oxidation Retrofit Program	-	-	-	-	-	-	-	-	-	-
5-6080-32	Filt.Plts., Distr. System, and Colorado River Aqueduct - Backflow Prevention Assemblies	616.9	646.6	642.9	-	-	-	-	-	-	-
5-6100-31	Jensen & Mills Filtration Plants - Oxidation Retrofit Program	-	-	-	-	-	-	-	-	-	-
5-5270-31	Jensen Filtration Plant - Expansion No. 1	-	-	-	-	-	-	-	-	-	-
5-0508-31	Jensen Filtration Plant - Repair Roof at Reservoir No. 1	-	-	-	-	-	-	-	-	-	-
5-5820-32	Jensen Filtration Plant - Replace Filter Media	-	-	-	-	-	-	-	-	-	-
5-6980-32	Jensen Filtration Plant - Sludge Handling Study	-	-	-	-	-	-	-	-	-	-
5-6860-31	Jensen Plant - Chemical Tank Farm Modifications	-	-	-	-	-	-	-	-	-	-
5-0112-63	La Verne Facilities - Construct a Utility Shop Building	-	-	-	-	-	-	-	-	-	-
5-0317-61	La Verne Facilities - Electrical Service Upgrade	-	-	-	-	-	-	-	-	-	-
5-6550-61	La Verne Facility - Hazardous Waste Staging Area	-	-	-	-	-	-	-	-	-	-
5-5570-31	Mills Filtration Plant - Expansion No. 2	-	-	-	-	-	-	-	-	-	-
5-0111-31	Mills Filtration Plant - Landfill	-	-	-	-	-	-	-	-	-	-
5-5610-21	San Joaquin Reservoir - Improvement	-	-	-	-	-	-	-	-	-	-
5-7010-11	San Joaquin Reservoir - Slope Repair (Met's Share)	-	-	-	-	-	-	-	-	-	-
5-6280-33	Skinner - Relocate Front Entrance Gate and Fencing, and Construct New Parking Lot	-	-	-	-	-	-	-	-	-	-
5-6110-31	Skinner Filtration Plant - Emergency Power Generating System	-	-	-	-	-	-	-	-	-	-
5-6660-31	Skinner Filtration Plant - Filter Media Replacement	-	-	-	-	-	-	-	-	-	-
5-0515-31	Skinner Filtration Plant - Flocculator Replacement in Modules 1 & 2	-	-	-	-	-	-	-	-	-	-
5-0304-31	Skinner Filtration Plant - Install Effluent Adjustable Weir	-	-	-	-	-	-	-	-	-	-
5-0410-31	Skinner Filtration Plant - Modules 4, 5, and 6 Sedimentation Basin	-	-	-	-	-	-	-	-	-	-
5-6310-31	Skinner Filtration Plant Monotill	-	-	-	-	-	-	-	-	-	-
5-6820-31	Skinner Modules 1-3 Electrical Conduit and Wireways Replace	-	-	-	-	-	-	-	-	-	-
5-0402-61	Warehouses and Storage Building At Mills Filtration Plant	-	-	-	-	-	-	-	-	-	-
5-0514-31	Water Quality - Cryptosporidium Action Plan	-	-	-	-	-	-	-	-	-	-
5-6590-31	Water Quality - Demonstration-Scale Testing	-	-	-	-	-	-	-	-	-	-
5-0401-61	Water Quality Lab - Inductively Coupled Plasma Mass Spectrometer	-	-	-	-	-	-	-	-	-	-
5-6350-63	Water Quality Laboratory Building Expansion	-	-	-	-	-	-	-	-	-	-
5-6910-32	Weymouth Filtration Plant - Sludge Handling Facility	-	-	-	-	-	-	-	-	-	-
5-6530-31	Weymouth Filtration Plant - Ferric Chloride Retrofit and Storage Augmentation	-	-	-	-	-	-	-	-	-	-
5-0002-32	Weymouth Replace Existing Asphalt Paving	-	-	-	-	-	-	-	-	-	-
TOTAL WATER QUALITY AND TREATMENT (EXISTING PLANTS)		616.9	646.6	642.9	-	-	-	-	-	-	-
5-5560-71	Central Pool Augmentation and Water Quality Project - Study and Land Acquisition	-	-	-	-	-	-	-	-	-	-
5-0221-32	Central Pool Augmentation Filtration Plant	13,602.3	16,219.6	15,209.6	15,370.8	73,900.0	100,864.5	104,654.3	96,341.2	-	-
	Central Pool Augmentation Filtration Plant - 2nd Stage	-	-	-	-	-	-	-	-	-	1,797.1
TOTAL CENTRAL POOL AUGMENTATION (Filtration Projects)		13,602.3	16,219.6	15,209.6	15,370.8	73,900.0	100,864.5	104,654.3	96,341.2	-	1,797.1
5-0516-31	Perris Filtration Plant	-	-	2,528.4	2,654.8	11,150.2	11,707.8	92,198.6	96,806.6	101,649.0	-
5-5800-71	Perris Filtration Plant - Study and Advance Land Acquisition	-	-	-	-	-	-	-	-	-	-
TOTAL PERRIS FILTRATION PLANT		-	-	2,528.4	2,654.8	11,150.2	11,707.8	92,198.6	96,806.6	101,649.0	-
SUBTOTAL FOR WATER TREATMENT PROJECTS		14,219.2	16,866.2	18,380.9	18,025.6	85,060.3	112,572.2	196,853.0	195,149.8	101,649.0	1,797.1
TOTAL PROPOSED CAPITAL IMPROVEMENT PROGRAM		82,008.4	92,811.3	96,519.0	97,226.4	167,231.5	346,952.5	437,422.0	464,023.5	336,566.5	81,396.1

Program No.	Title	2016-16	2016-17	2017-18	2018-19	2019-20	9596-19/20 Total	Program Costs w/o Cont.	Cont.	Total Program Costs With Cont.					
WATER TREATMENT PROJECTS															
5-6270-61	All Facilities - Discharge Elimination	-	-	-	-	-	2,212.9	2,215.1	-	2,215.1					
5-6030-31	All Filtration Plants and Distribution System - Chemical Spill Containment	-	-	-	-	-	10,289.1	28,418.3	9,583.7	29,000.0					
5-0122-33	Diemer - Relocate Front Entrance Gate and Install Lighting, Sec Camera and Gate Control	-	-	-	-	-	779.0	780.7	110.8	891.5					
5-6810-31	Diemer and Weymouth Install Emergency Generators	-	-	-	-	-	3,304.5	3,828.4	508.0	4,337.4					
5-6820-31	Diemer Chemical Feed Pumps Relocation	-	-	-	-	-	894.0	1,231.1	216.0	1,447.1					
5-6760-31	Diemer Filtration Plant - Chlorination System Modifications	-	-	-	-	-	2,534.2	3,515.0	320.9	3,836.0					
5-0503-31	Diemer Filtration Plant - Construct Sedimentation Basin Spillway	-	-	-	-	-	2,831.8	2,831.8	416.3	3,248.1					
5-0509-31	Diemer Filtration Plant - Domestic Water System Improvement	-	-	-	-	-	840.2	873.8	131.5	1,005.1					
5-6570-31	Diemer Filtration Plant - Mixing and Settling Basin No. 8 - North Slope Remedation	-	-	-	-	-	1,368.5	1,812.4	178.6	1,991.0					
5-0501-31	Diemer Filtration Plant - New Finished Water Reservoir	-	-	-	-	-	83,864.7	83,705.9	7,513.4	81,193.3					
5-0502-61	Diemer Filtration Plant - New Maintenance Building	-	-	-	-	-	3,837.1	3,837.1	548.5	4,385.6					
5-6990-11	Diemer Filtration Plant - Upgrade Flocculator Drives	-	-	-	-	-	330.3	362.2	67.8	430.0					
5-5500-31	Diemer Filtration Plant Modifications and Washwater Reclamation Plant Enlargement	-	-	-	-	-	1,869.3	27,840.3	669.7	28,500.0					
5-6640-31	Diemer Land Acquisition and Habitat Conservation Plan	-	-	-	-	-	5,390.8	5,880.1	-	5,880.1					
5-0250-31	Diemer, Weymouth & Skinner Filtration Plants - Oxidation Retrofit Program	-	-	-	-	-	306,785.4	306,785.4	64,966.2	371,751.6					
5-6080-32	FILPits, Distr. System, and Colorado River Aqueduct - Backflow Prevention Assemblies	-	-	-	-	-	13,619.1	14,625.8	2,874.1	17,500.0					
5-6100-31	Jensen & Mills Filtration Plants - Oxidation Retrofit Program	-	-	-	-	-	165,469.3	176,639.7	23,360.4	200,000.1					
5-5270-31	Jensen Filtration Plant - Expansion No. 1	-	-	-	-	-	18,407.3	182,764.9	2,245.1	185,000.0					
5-0508-31	Jensen Filtration Plant - Repair Roof at Reservoir No. 1	-	-	-	-	-	1,412.7	1,428.9	171.1	1,600.0					
5-5820-32	Jensen Filtration Plant - Replace Filter Media	-	-	-	-	-	-	778.9	-	778.9					
5-6980-32	Jensen Filtration Plant - Sludge Handling Study	-	-	-	-	-	302.8	314.3	35.7	350.0					
5-6860-31	Jensen Plant - Chemical Tank Farm Modifications	-	-	-	-	-	147.9	390.9	60.7	451.5					
5-0112-63	La Verne Facilities - Construct a Utility Shop Building	-	-	-	-	-	8,306.0	8,306.0	1,186.4	9,491.4					
5-0317-61	La Verne Facilities - Electrical Service Upgrade	-	-	-	-	-	878.8	621.9	83.5	760.3					
5-6550-61	La Verne Facility - Hazardous Waste Staging Area	-	-	-	-	-	2,017.1	2,082.2	260.3	2,312.5					
5-5570-31	Mills Filtration Plant - Expansion No. 2	-	-	-	-	-	46,167.0	137,394.6	22,606.4	160,000.0					
5-0111-31	Mills Filtration Plant - Landfill	-	-	-	-	-	7,895.8	7,896.6	-	7,896.6					
5-5610-21	San Joaquin Reservoir - Improvement	-	-	-	-	-	21,316.0	24,641.2	3,128.8	27,770.0					
5-7010-11	San Joaquin Reservoir - Slope Repair (Mef's Share)	-	-	-	-	-	494.1	529.1	70.9	600.0					
5-6280-33	Skinner - Relocate Front Entrance Gate and Fencing, and Construct New Parking Lot	-	-	-	-	-	239.8	322.3	69.3	391.5					
5-6110-31	Skinner Filtration Plant - Emergency Power Generating System	-	-	-	-	-	1,816.6	2,291.1	362.9	2,664.0					
5-6660-31	Skinner Filtration Plant - Filter Media Replacement	-	-	-	-	-	6,327.2	6,491.7	1,051.5	6,843.2					
5-0515-31	Skinner Filtration Plant - Flocculator Replacement in Modules 1 & 2	-	-	-	-	-	1,804.2	1,804.2	185.8	1,990.0					
5-0304-31	Skinner Filtration Plant - Install Effluent Adjustable Weir	-	-	-	-	-	691.7	691.7	98.8	790.4					
5-0410-31	Skinner Filtration Plant - Modules 4, 5, and 6 Sedimentation Basin	-	-	-	-	-	39,198.4	39,198.4	5,599.8	44,798.2					
5-6510-31	Skinner Filtration Plant Monofill	-	-	-	-	-	1,208.0	1,810.6	176.3	1,984.9					
5-6920-31	Skinner Modules 1-3 Electrical Conduit and Wireways Replace	-	-	-	-	-	443.8	704.7	57.3	762.0					
5-0402-61	Warehouse and Storage Building At Mills Filtration Plant	-	-	-	-	-	2,338.5	2,362.4	237.6	2,600.0					
5-0514-31	Water Quality - Cryptosporidium Action Plan	-	-	-	-	-	5,033.6	5,033.6	603.2	5,636.8					
5-6590-31	Water Quality - Demonstration-Scale Testing	-	-	-	-	-	5,091.6	9,133.0	131.4	9,264.4					
5-0401-61	Water Quality Lab - Inductively Coupled Plasma Mass Spectrometer	-	-	-	-	-	284.4	301.0	28.5	329.5					
5-6350-63	Water Quality Laboratory Building Expansion	-	-	-	-	-	11,964.8	12,906.5	1,306.0	14,811.5					
5-6910-32	Weymouth Filtration Plant - Sludge Handling Facility	-	-	-	-	-	5,174.3	5,449.1	580.9	6,030.0					
5-6530-31	Weymouth Filtration Plant - Ferric Chloride Retrofit and Storage Augmentation	-	-	-	-	-	1,311.2	1,358.6	222.2	1,577.8					
5-0002-32	Weymouth Replace Existing Asphalt Paving	-	-	-	-	-	906.0	917.3	266.2	1,183.5					
TOTAL WATER QUALITY AND TREATMENT (EXISTING PLANTS)							762,135.9	1,092,243.8	163,080.7	1,245,324.5					
5-5560-71	Central Pool Augmentation and Water Quality Project - Study and Land Acquisition	-	-	-	-	-	22,994.1	40,248.6	-	40,248.6					
5-0221-32	Central Pool Augmentation Filtration Plant	-	-	-	-	-	438,162.4	438,162.4	69,214.8	497,377.1					
	Central Pool Augmentation Filtration Plant - 2nd Stage	3,774.0	3,962.7	31,206.2	32,766.5	34,404.8	107,911.4	107,911.4	-	107,911.4					
TOTAL CENTRAL POOL AUGMENTATION (Filtration Projects)							3,774.0	3,962.7	31,206.2	32,766.5	34,404.8	689,067.9	698,320.1	89,214.8	645,534.9
5-0516-31	Perms Filtration Plant	-	-	-	-	-	318,887.4	318,887.4	41,327.0	360,214.3					
5-5800-71	Perms Filtration Plant - Study and Advance Land Acquisition	-	-	-	-	-	19,387.4	20,558.2	-	20,558.2					
TOTAL PERMS FILTRATION PLANT							338,274.8	339,445.6	41,327.0	380,772.5					
SUBTOTAL FOR WATER TREATMENT PROJECTS		3,774.0	3,962.7	31,206.2	32,766.5	34,404.8	1,689,286.5	2,017,816.4	263,622.4	2,271,438.9					
TOTAL PROPOSED CAPITAL IMPROVEMENT PROGRAM		80,063.2	97,412.1	129,328.1	200,113.1	210,118.7	7,867,381.4	8,230,431.1	835,010.0	9,068,441.1					

**SOUTHERN CALIFORNIA'S
INTEGRATED WATER RESOURCES PLAN**

**APPENDIX F:
IRPSIM MODEL DESCRIPTION**

Report No. 1107

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APPENDIX F:

IRPSIM MODEL DESCRIPTION

BACKGROUND

The primary goals of the modeling for the Integrated Resources Planning process were: (1) to determine the probability of regional water supply surplus or shortage, and (2) to define resources that could contribute to meeting a regional supply reliability goal. A simulation modeling technique was chosen to accomplish these goals, because simulation is highly effective in determining the probabilistic outcomes. In addition, simulation allows for flexibility in defining the variables needed for a scenario-based analysis over a long planning horizon, and provides a mechanism for including stochastic uncertainty in forecasts of supply and demand.

Specifically, the Integrated Resources Planning Simulation Model (IRPSIM) uses a sequentially-indexed Monte Carlo simulation algorithm to simulate future supply surplus/shortage conditions using correlated hydrologic variations in regional supplies and demands. In using this type of simulation algorithm, well defined operational rules for supply and storage operations are employed to meet the objectives of the simulation. The sequentially-indexed Monte Carlo process applies historical effects of hydrology and weather to forecasts of supplies and demands, generating a distribution of projected surplus/shortage conditions. This appendix contains definitions of the variables and ratios used in IRPSIM, the objectives of the IRPSIM algorithm, a description of the simulation processes (supply and demand, and storage operations), and an example of the storage algorithm used in IRPSIM.

VARIABLES AND RATIOS

Although many individual variables are used in IRPSIM, only the ones critical for understanding its algorithm will be defined.

- Demand:** The aggregate retail-level demand for water.
- Supply:** The aggregate water supply from all sources, local and imported.
- Surplus/Shortage:** The contemporaneous surplus or shortage of water, *Supply-Demand*, before storage puts or takes. Surpluses are represented as positives, shortages as negatives.
- Storage Device:** A groundwater basin or surface reservoir.

In-Lieu

Conveyance: The ceiling on the amount of in-lieu deliveries that a groundwater basin can and/or will take. In-lieu deliveries to a storage device are made by reducing groundwater pumping below safe yield for any single time step. The reduced pumping allows the basin to fill by accumulating natural runoff or regular replenishment.

Put/Take: The put or take from a storage device, or aggregate of all storage devices. Puts are represented as positives, takes as negatives.

Net-Surplus/

Net-Shortage: The surplus or shortage of water after storage puts and takes. Surpluses are represented as positives, shortages as negative.

Storage Capacity: The total space in a storage device dedicated to storing water for regional purposes. Storage capacity can be defined for an individual storage device or for the aggregate of all storage devices.

Put Conveyance: The physical spreading and/or injection capacity of a storage device.

Take Conveyance: The physical pump or withdrawal capacity of a storage device. (for groundwater basins, this is derived as the maximum production capacity minus groundwater production).

Storage Level: The total amount of water stored in a storage device at a particular time step.

Remaining

Storage Capacity: The *storage capacity* minus *storage level* for a storage device. Remaining storage capacity varies with time due to changes in storage level and storage capacity.

Put Ratio: The minimum number of time steps required to fill the *Remaining Storage Capacity* of a storage device, provided there is enough water supply to maximize *Put Conveyance*. Mathematically, this variable is equal to *Remaining Storage Capacity* divided by *Put Conveyance*.

Overlying

Demand: The aggregate water demands of Metropolitan Water District's Member Agencies, Sub-Agencies, or Retailers, minus their respective local supplies, that overlies any single groundwater basin. This variable is interpreted as the maximum potential storage take for a groundwater basin, without export of the water to another region, or as the demand for imported water within the area of service for a groundwater basin.

Modified

Take Conveyance: The maximum *take conveyance* for which there is an *overlying demand*. This variable is equal to the lesser of *take conveyance* or *overlying demand*.

Take Ratio: The minimum number of time steps required to empty a storage device given its *Storage Level*, provided there is enough water demand from which to maximize the *Modified Take Conveyance*. Mathematically this variable is equal to *Storage Level* divided by *Take Conveyance*.

OBJECTIVES

There are four objectives for the IRPSIM algorithm: (1) meet consumptive demands for water with coincident water production, (2) minimize the amount of wasted water; (3) efficiently use storage withdrawals to alleviate shortages; and (4) prioritize storage operations to fill storage: local (Groundwater & Surface), regional, and then outside service area. The four objectives split the IRPSIM algorithm into two separate parts; the production of supply and demand (objective 1), and the operation of storage (objectives 2-4).

Objective 1 has top priority in the IRPSIM algorithm, and also determines the supply surplus / shortage conditions used by the storage algorithm. Ideally, Objectives 2-4 would not be prioritized, so that all would carry the same importance. However, Objectives 2-4 are often in competition with each other. For example, in order to minimize wasted water, surplus water should be stored so as to maximize the likelihood of having remaining put conveyance in the future. In other words, when you have a choice between two groundwater basins to store surplus water, the groundwater basin with the lowest ratio of remaining storage capacity divided by its put conveyance should be used. This metric, called the *put ratio*, can help govern storage put decisions. In particular, the *put ratio* is interpreted as the number of future time steps required to fill the remaining storage, if there is ample water. Choosing where to store surplus water by *put ratio* assures that the maximum amount of put conveyance and remaining storage capacity is available in the future. However, this ratio conflicts with the objective of storing water to maximize future storage production. To accomplish this objective, surplus water should be stored in the basin with the lowest ratio of storage level divided by its take conveyance. This metric, called the *take ratio*, is interpreted as the number of time steps required to empty a storage device. These ratios can sometimes suggest alternative storage rules depending on the objective chosen. Therefore, objectives sometimes need to be prioritized.

The IRPSIM algorithm is most easily understood when broken into two parts: (1) The generation of future supplies and demands, and (2) the routing and balancing of storage.

SUPPLY AND DEMAND GENERATION

Future supplies and demands are generated by IRPSIM using equations specified in the variable definition (VARDEF) file. The VARDEF file is IRPSIM's primary source for data inputs and provides a flexible variable language for manipulating input data. IRPSIM is not a forecasting model. It is a tool for integrating supply and demand forecasts from several sources and creating an estimation of water supply reliability. The actual forecasts of supply and demand data must come from other models. IRPSIM uses an internal algorithm to cycle the effect of historical hydrologies on both supply and demand to estimate the impacts of weather variation on supply reliability. IRPSIM is also capable of generating and applying a random error term to both supplies and demands to reflect uncertainty in forecasted data.

IRPSIM equations allow for the combination of data from several non-integrated models. In this way, IRPSIM can leverage the information from multiple data sources. For example, MWD's long-range demand forecasting model, MWD-MAIN, produces weather normal forecasts, but does not have weather effects applied to its forecasts. However, weather effects are available from MWD's short-range demand forecast tool, MWD-FORE. By combining these two data sources, IRPSIM produces a "hybrid" demand forecast consisting of long-range trends and short-range weather variability in its demand projections. In this same way, IRPSIM combines data for all supply and demand data to create aggregate demand and supply.

IRPSIM uses an innovative approach called indexed-sequential monte-carlo simulation to evaluate supplies and demands. Indexed simulation means that imported supplies from Northern California and the Colorado River are indexed to the same historical year as local demand and supplies in Southern California. This methodology preserves the contemporaneous relationships between hydrology and climate effects on supply and demand. In other words, 1933's weather impact on Northern California's hydrology is matched with 1933's weather impact on demands and local supplies in Southern California and so forth for all supplies and demands. The indexing between supply and demand is critical because of the relationship between the two. The demand for water is inversely correlated with the supply. The same factors that tend to make demand increase (hot and dry weather), also tend to decrease supply availability.

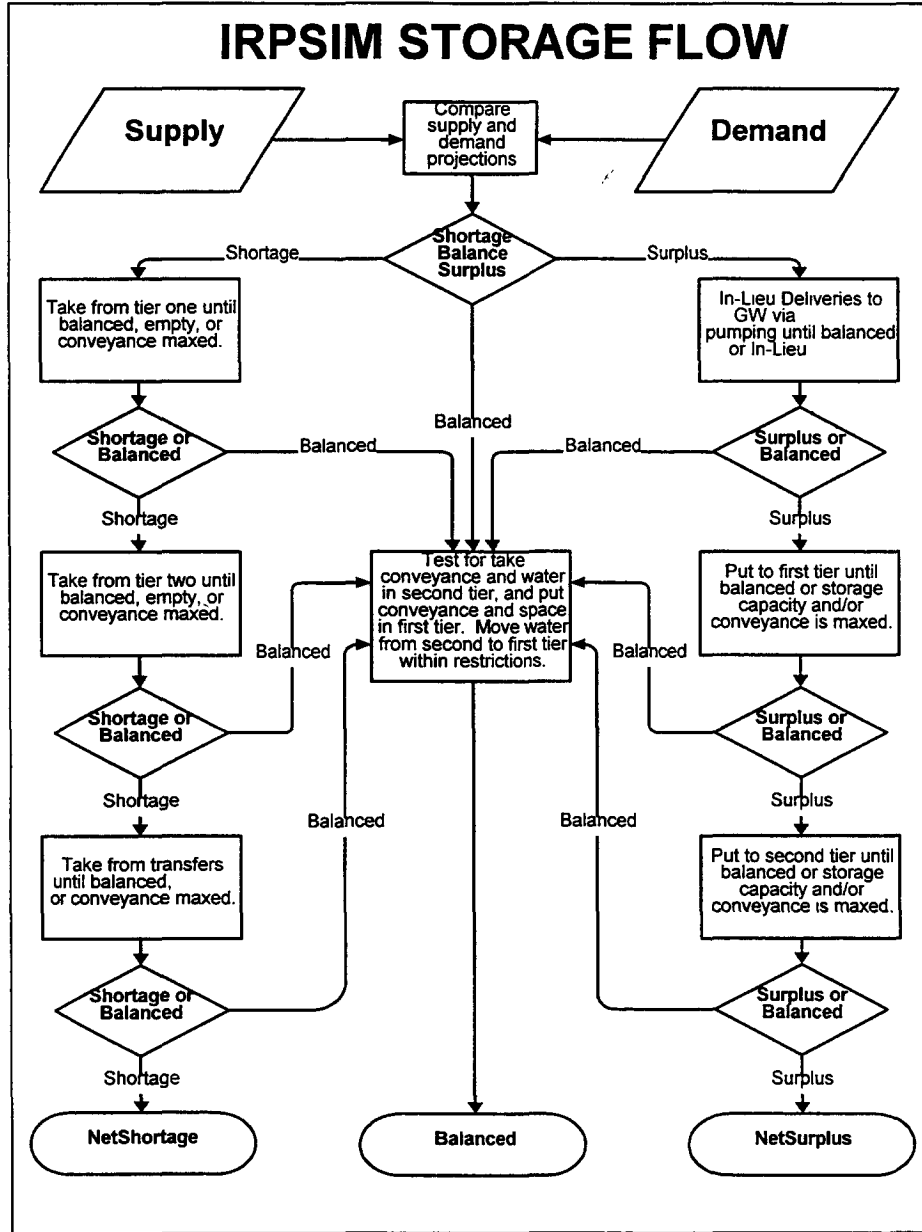
The simulation approach not only preserves the match between supply and demand, but also the sequence of years. Sequential simulation (preserving the order of the historical year's climate and hydrology) can identify the times in which demands exceed supplies and vice versa. This analysis is critical for determining storage needs. In addition, sequential simulation preserves the interrelationship of weather between years. Statistical models that are used to generate the weather effect on water demand, or hydrology effect on water supply, generally measure a multi-year effect. This means that the estimate of a weather effect on demand is based on the previous two or three year's weather. The same is true for hydrologic models of supply. Therefore, if 1987 were separated from 1984, 1985 and 1986 in the sequence, then the estimated weather or hydrology effect would not be valid.

The sequentially indexed monte-carlo method developed for IRPSIM is best described in its simplest form. Assume that water supply and demand come from independent distributions. Simply by taking a random draw from each distribution and subtracting them (supply minus demand), and repeating this hundreds of times, a distribution of shortage/surplus can be constructed. However, this simplified method is complicated by the negative correlation between supply and demand. Therefore, in order to determine supply reliability for water, matched pairs of supply and demand must be used to develop the distribution of shortage/surplus. Matching pairs of supply and demand, a low likelihood that a low demand observation gets paired with a low supply observation. IRPSIM combines the indexed-sequential simulation discussed earlier with Monte-Carlo probability analysis in order to obtain the final distribution of shortage/surplus used to estimate supply reliability. The model takes each of the unique 70 year climate/hydrology traces in the historical record (from 1922-1991) and draws about 28 different random non-weather related demands. This provides about 2,000 individual events for any specified time-step (usually monthly).

THE ROUTING AND BALANCING OF STORAGE

The basic flow rules for storage in IRPSIM are depicted in Figure F-1 below.

Figure F-1



In step one, total supply and demand are compared to determine if there is surplus or shortage. (or the unlikely outcome of exact balance). Based on this determination, water is either put to or taken from storage. If there is a surplus, water is delivered in-lieu to the groundwater basins until the surplus is depleted or until the in-lieu conveyance reaches its maximum. Any

additional surplus water is put into tier one storage: groundwater basins¹, Lake Mathews, a San Diego surface reservoir, and Emergency Eastside Reservoir, up to the put conveyance or storage capacity of tier one. If surplus water remains, it is put into tier two storage: Non-Emergency Eastside Reservoir (the carryover portion). Any remaining surplus (net-surplus) is unusable in the Metropolitan Service Area, and is left as surplus on the State Water Project (or it could be used in yet undefined storage transfer facilities on the SWP). If the initial condition is shortage, then water is taken from tier one first (excluding Emergency Eastside Reservoir²). If shortage remains then water is taken from tier two storage. If shortages still exist then State Water Project Transfers are called. Finally, any remaining shortage (net-shortage) is true retail-level shortage and is counted against the region's reliability goal.

The linkage to the center line of the chart, the balanced path, represents an attempt to move water from Eastside Reservoir (Non-Emergency) into tier one storage. This movement of water, or storage shift, is attempted whenever there is surplus conveyance between Eastside Reservoir and tier one storage. Storage shift serves two purposes: (1) it transfers water closer to ultimate water demand off-peak, reducing the need for peak facilities; and (2) it frees up storage space in Eastside Reservoir to receive hydrologic or unexpected surpluses from the Colorado River Aqueduct or the State Water Project, reducing the overall likelihood of unused surplus water (net-surplus). In simulation, the storage shift rules allow groundwater basins to use their spreading basins in the winter for natural runoff while Eastside Reservoir fills, then receive deliveries from Eastside Reservoir in late spring or summer when there is spreading capacity available.

These gross flow rules handle a majority of the decisions for storage in IRPSIM. However, they do not address issues regarding the placement of water within a tier. For example, if there is only enough surplus to put water into a few tier one facilities, which facilities get the water? Conversely, if there is a shortage requiring storage takes from only a few tier one storage devices then which devices are used? In order to make these decisions, objectives of the storage algorithm had to be prioritized, and an optimal storage rule had to be developed³.

As stated above, the objective of minimizing net-surplus and the objective of maximizing potential takes (which is equivalent to minimizing net-shortage), are sometimes in conflict. This conflict arises whenever a choice between tier one storage devices must be made. To fully understand this conflict, examine the following examples in which only two storage devices exist. In Example 1, shown in Table F-1, storage is balanced based on take ratios (putting and taking water from storage so that take ratios are as equal as possible across all storage devices within a tier). Balancing storage by take ratios maximizes the efficiency of future storage takes.

¹ Metropolitan Water District to Member Agency connections, specifically designed for groundwater spreading and/or injection, allow groundwater deliveries over and above the ceiling of in-lieu deliveries. Additionally, the configuration of most Member Agencies precludes delivery of in-lieu water to portions of their retail demand, allowing a substantial remainder of groundwater conjunctive use potential to only be accessible through tier one (direct) deliveries.

² Emergency Eastside Reservoir never experiences a take unless a catastrophic emergency has occurred (an aqueduct severing earthquake).

³ The Single Step Optimal Storage Rule documented below was developed for the MWD IRP process and is documented here for the first time.

By the end of six months, both storage devices have 3 months of maximum storage take available (storage level divided by modified storage take)⁴. Therefore, if three months of shortage were to occur, the storage devices would have enough water in storage and take conveyance to maximize takes. However, there is a drawback to this approach. If the next three months had large surpluses then storage device 2 would be full in 2.3 months. This would effectively

Table F-1

Example 1						
Month	1	2	3	4	5	6
Supply	1200	1300	1200	1000	1000	1000
Demand	1000	900	1000	1100	1100	1200
Surplus/Shortage	200	400	200	-100	-100	-200
Net-Surplus/Net-Shortage	0	0	0	0	0	0
Device 1						
Storage Capacity	1000	1000	1000	1000	1000	1000
Storage Level	100	155	305	390	355	300
Remaining Storage Capacity	900	845	695	610	645	700
Put Conveyance	150	150	150	150	150	150
Take Conveyance	100	100	100	100	100	100
Overlying Demand	90	81	90	99	99	108
Modified Take Conveyance	90	81	90	99	99	100
Take Ratio	1.1	1.9	3.4	3.9	3.6	3.0
Put Ratio	6.0	5.6	4.6	4.1	4.3	4.7
Put/Take	55	150	85	-35	-55	-75
Device 2						
Storage Capacity	1200	1200	1200	1200	1200	1200
Storage Level	100	245	495	610	545	500
Remaining Storage Capacity	1100	955	705	590	655	700
Put Conveyance	300	300	300	300	300	300
Take Conveyance	250	250	250	250	250	250
Overlying Demand	140	126	140	154	154	168
Modified Take Conveyance	140	126	140	154	154	168
Take Ratio	0.7	1.9	3.5	4.0	3.5	3.0
Put Ratio	3.7	3.2	2.4	2.0	2.2	2.3
Put/Take	145	250	115	-65	-45	-125

⁴ Put and take ratio are actually beginning period variables, meaning that they are based on the actions of the previous period. Therefore, the ratio of true interest is calculated for month seven, and is not displayed in the chart. The balance that appears in month six is based on the actions of month 5.

Table F-2

Example 2						
Month	1	2	3	4	5	6
Supply	1200	1300	1200	1000	1000	1000
Demand	1000	900	1000	1100	1100	1200
Surplus/Shortage	200	400	200	-100	-100	-200
Net-Surplus/Net-Shortage	0	0	0	0	0	0
Device 1						
Storage Capacity	1000	1000	1000	1000	1000	1000
Storage Level	100	250	400	550	550	533
Remaining Storage Capacity	900	750	600	450	450	467
Put Conveyance	150	150	150	150	150	150
Take Conveyance	100	100	100	100	100	100
Overlying Demand	90	81	90	99	99	108
Modified Take Conveyance	90	81	90	99	99	100
Take Ratio	1.1	3.1	4.4	5.6	5.6	5.3
Put Ratio	6.0	5.0	4.0	3.0	3.0	3.1
Put/Take	150	150	150	0	-17	-66
Device 2						
Storage Capacity	1200	1200	1200	1200	1200	1200
Storage Level	100	150	400	450	350	267
Remaining Storage Capacity	1100	1050	800	750	850	933
Put Conveyance	300	300	300	300	300	300
Take Conveyance	250	250	250	250	250	250
Overlying Demand	140	126	140	154	154	168
Modified Take Conveyance	140	126	140	154	154	168
Take Ratio	0.7	1.2	2.9	2.9	2.3	1.6
Put Ratio	3.7	3.5	2.7	2.5	2.8	3.1
Put/Take	50	250	50	-100	-83	-134

reduce the put conveyance of storage to that in storage device 1. The alternative, Example 2 (illustrated in Table F-2), is to balance storage by put ratios. Balancing storage by put ratios maximizes the efficiency of future storage puts. Therefore, if the next three months had large surpluses then there would be enough remaining storage capacity to maximize storage puts for all three months. The drawback of Example 2 is reflected in the take ratios. If there were three severe shortage months ahead, then device 2 would be empty in 1.6 months, effectively reducing overall take conveyance to that of device 1. The fundamental question is whether it is more important to minimize unused surplus or to minimize shortage. Since the IRP process was initiated to address supply reliability, it was decided to use the take ratio method and focus on minimizing shortage.

The take ratio rule is used at any point in the IRPSIM storage algorithm where there is less shortage than take conveyance and storage level available, or when there is less surplus than put conveyance of remaining storage capacity available. The take rule is applied whenever there is less storage shift than remaining put conveyance and remaining storage capacity in tier one. After storage has been resolved for all shortages and surpluses, there may be remaining ability for storage shift (movement of water from Eastside Reservoir to tier one storage). When this occurs, it may be necessary to prioritize this shift for tier one deliveries; if there is not enough water in storage shift from Eastside Reservoir to meet all the remaining put conveyance or remaining storage capacity in tier one.

A STORAGE EXAMPLE

The following, Table F-3, shows an example of the storage algorithm. Only three storage devices are assumed to exist: two tier one storage devices and one tier two storage device. For simplicity, no in-lieu conveyance is assumed. However, in-lieu operation can be surmised from the example. Supplies and demand are as given, and tier one is balanced using the take rule.

Table F-3

Month	1	2	3	4	5	6	7	8	9	10	11	12
Supply	1700	1700	1600	1500	1200	1100	1000	1050	1200	1300	1400	1500
Demand	900	800	1000	1100	1300	1400	1400	1300	1100	1000	900	900
Surplus/Shortage	800	900	600	400	-100	-300	-400	-250	100	300	500	600
Net-Surplus/Net-Shortage	0	100	0	0	0	0	0	0	0	0	0	0
TIER 1												
Device 1												
Storage Capacity	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200	1200
Storage Level	100	250	400	550	700	725	710	710	642	722	842	992
Remaining Storage Capacity	1100	950	800	650	500	475	490	490	558	478	358	208
Put Conveyance	150	150	150	150	150	150	150	150	150	150	150	150
Take Conveyance	100	100	100	100	100	100	100	100	100	100	100	100
Overlying Demand	81	72	90	99	117	126	126	117	99	90	81	81
Modified Take Conveyance	81	72	90	99	100	100	100	100	99	90	81	81
Take Ratio	1.2	3.5	4.4	5.6	7.0	7.3	7.1	7.1	6.5	8.0	10.4	12.2
Put/Take	150	150	150	130	-60	-100	-100	-68	80	120	150	150
Storage Shift	0	0	0	20	85	85	100	0	0	0	0	0
Device 2												
Storage Capacity	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800	1800
Storage Level	100	400	700	1000	1300	1425	1390	1290	1108	1128	1308	1608
Remaining Storage Capacity	1700	1400	1100	800	500	375	410	510	692	672	492	192
Put Conveyance	300	300	300	300	300	300	300	300	300	300	300	300
Take Conveyance	250	250	250	250	250	250	250	250	250	250	250	250
Overlying Demand	126	112	140	154	182	196	196	182	154	140	126	126
Modified Take Conveyance	126	112	140	154	182	196	196	182	154	140	126	126
Take Ratio	0.8	3.6	5.0	6.5	7.1	7.3	7.1	7.1	7.2	8.1	10.4	12.8
Put/Take	300	300	300	270	-40	-196	-196	-182	20	180	300	300
Storage Shift	0	0	0	30	165	161	96	0	0	0	0	0
TIER 2												
Device 1												
Storage Capacity	800	800	800	800	800	800	800	800	800	800	800	800
Storage Level	0	350	700	850	800	550	300	0	0	0	0	50
Remaining Storage Capacity	800	450	100	-50	0	250	500	800	800	800	800	750
Put Conveyance	350	350	350	350	350	350	350	350	350	350	350	350
Take Conveyance	250	250	250	250	250	250	250	250	250	250	250	250
Put/Take	350	350	150	0	0	-4	-104	0	0	0	50	150
Storage Shift	0	0	0	-50	-250	-246	-196	0	0	0	0	0

In month one, with a surplus of 800 AF, all storage is at its maximum put conveyance, and water is stored in all three devices apparently equally. Likewise in month two all put conveyance is utilized, but 100 AF is left as net-surplus. In month three it becomes apparent that tier one storage has preference for water over tier two, because its put conveyance is maximized, before tier two receives water. No balance rules have been used to this point, because there hasn't been a case when there wasn't enough water to maximize all tier one put conveyance. In month four the surplus is smaller than the combined put conveyance of tier one, so the take rule for balancing storage is applied⁵. Next, water is shifted from tier two to tier one. This is possible because the put conveyance of tier one has not been maximized by

⁵ Although the rule is named the Take Rule, it is applied during puts and takes. The rule name comes from the ratio it uses; not from when it is applied.

direct puts, and take conveyance of tier two has not been maximized by demand. Since there is enough water being shifted to maximize tier one puts (device 1: direct put of 130 AF and shift of 20 AF, and device 2: direct put of 270 AF and shift of 30 AF), storage balancing is not employed⁶. Month 5 has the first shortage month, and takes are balanced among tier one storage. The shift is balanced as well because tier one put conveyance is not maximized by the maximum tier two shift (equal to tier two's maximum take conveyance). The balancing that occurs is evidenced by the equal take ratios in month 6 (see footnote 4 above). Also in month 6, the modified take conveyance of device 2 forces a direct take from tier two. This implies that the shortage in month 6, although smaller than the overall take conveyance of tier one, was not distributed according to conveyance. Therefore, meeting this shortage solely out of tier one storage would require export facilities that are not assumed in the IRPSIM runs. Storage shift continues to keep tier one in balance until month 8, because tier two take conveyance never maximizes tier one put conveyance⁷.

Although the example above is greatly simplified, having only two tier one devices and no in-lieu capabilities, it illustrates several important features of the storage algorithm. First, no water is put into tier two storage devices, unless it is unusable by tier one storage devices. Second, tier one is optimized for minimizing future shortages, using the heuristics of the take ratio rule. Third, storage is moved from tier two to tier one whenever possible. Fourth, tier one takes are restricted to meeting the demand for Metropolitan water that overlies the particular storage device.

⁶ It is also important to realize that any shift that maximizes put conveyance of tier one, negates the balancing that occurred for direct puts in that month. However, it is still necessary to balance direct puts whenever possible, because it is impossible to know a priori whether storage shift will maximize put conveyance.

⁷ Following this logic it may seem impossible for a tier two storage device to ever maximize tier one storage (given the relative sizes and conveyances), but it can happen as preferred tier one storage devices fill, effectively decreasing the put conveyance of tier one.

**SOUTHERN CALIFORNIA'S
INTEGRATED WATER RESOURCES PLAN**

**APPENDIX G:
SUPPLY RELIABILITY AND LEAST-COST PLANNING**

Report No. 1107

January, 1996

APPENDIX G:

SUPPLY RELIABILITY AND LEAST-COST PLANNING

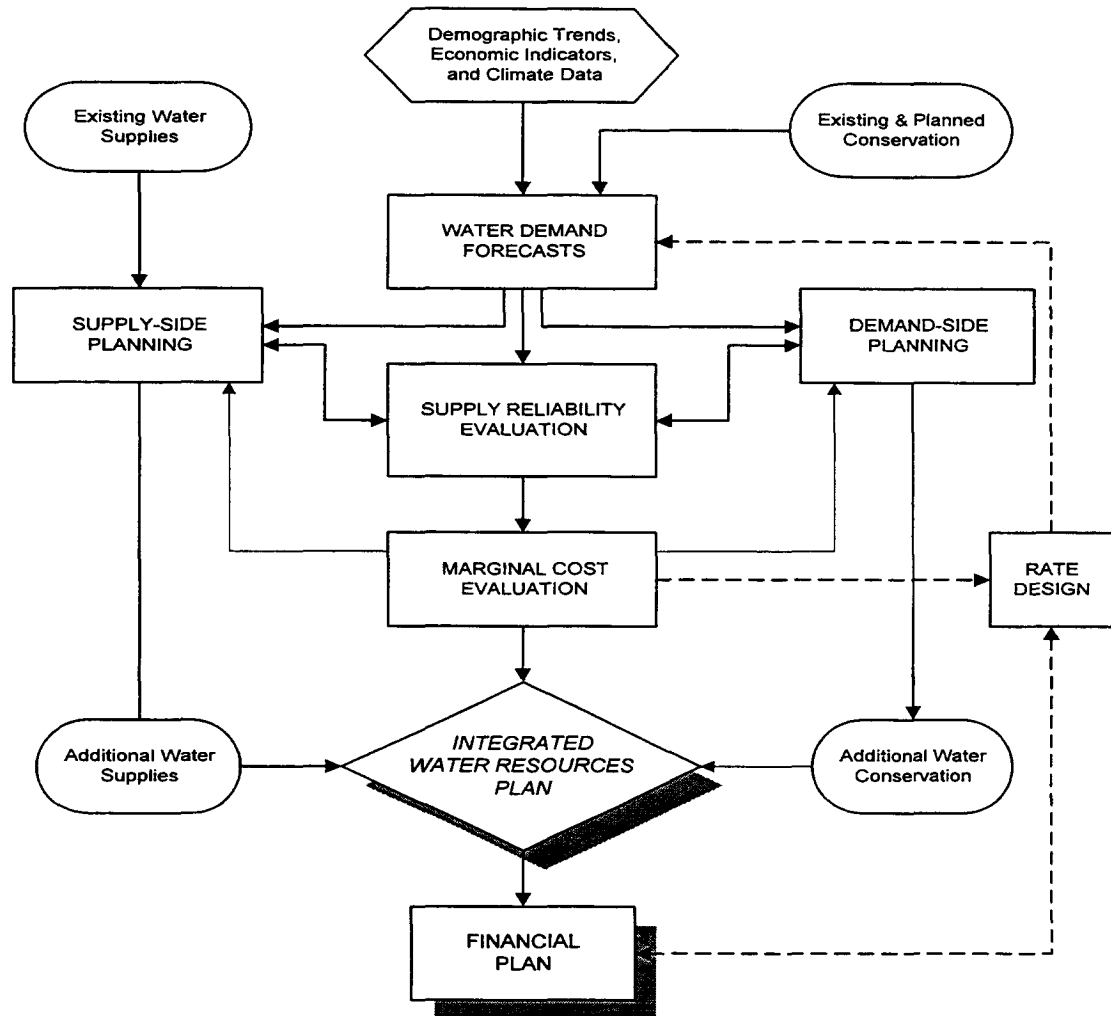
Traditionally, water supply planning has been fairly straightforward -- emphasizing the construction of supply projects such as surface reservoirs, treatment plants, wells and pipelines to meet growing demands. However, due to rising capital costs, increased environmental and water quality regulations, and attendant competition for new water supplies, different approaches to traditional supply planning must be used. These new planning approaches can be adapted from the techniques used by the power industry, such as least-cost planning (LCP) and integrated resource planning (IRP). In general, LCP is a procedure that compares the costs (resource development and environmental externalities) of traditional supply projects with demand-side management programs (conservation). Based on the principle of minimizing costs, the combination of supply options and demand-side management with the lowest overall cost should be pursued. IRP is a dynamic planning process which incorporates the basic principles of LCP, and explicitly considers other objectives such as environmental protection, sustainable growth, and the economy (Beecher, et al., 1991). Although traditional supply planning as often involved analysis of supply reliability, both LCP and IRP require detailed reliability evaluations which take into account non-traditional resources.

Even though IRP's will differ for each water utility due to the unique characteristics of its service area, there are some basic technical steps that should be followed:

1. Develop a Detailed Water Demand Forecast
2. Estimate Current and Future Water Supplies
3. Estimate the Variation in Demands and Supplies Due to Weather & Hydrology
4. Estimate the Effectiveness of Demand-Side Management
5. Estimate the Cost of Water Supplies and Demand-Side Management
6. Assess the Risk Associated with the Development of Supplies and Demand-Side Management

This technical appendix summarizes the analytical techniques used ^{to} analyze supply reliability and develop the appropriate resource targets for local and imported supplies. It details the theory and principles of supply reliability planning and least-cost planning that were used for the IRP. Figure G-1 presents a general flow chart of the technical evaluations that should be incorporated into an IRP.

**Figure G-1
Technical Steps in Developing an IRP**



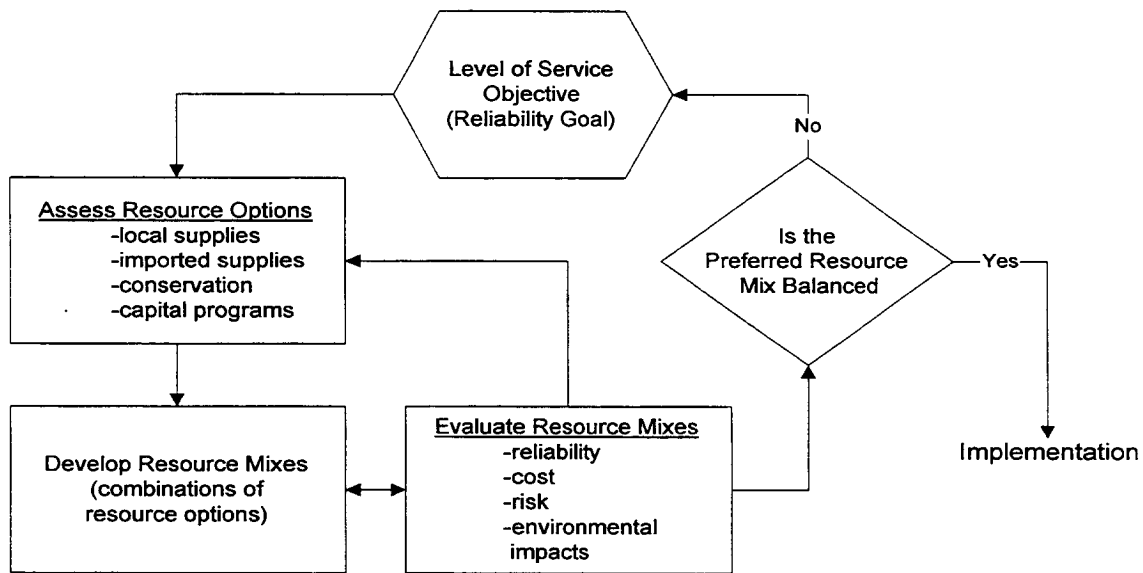
Metropolitan's IRP process started with the adoption of a water supply reliability goal, which states:

Through the implementation of the Integrated Resources Plan, Metropolitan and its member agencies will have the full capability to meet full service demands at the retail level under all foreseeable hydrologic events.

One of the major objectives of the IRP was to determine whether this goal was attainable and affordable. To determine whether the reliability goal was appropriate, a technical process was developed to analyze different resource strategies in a systematic fashion. Figure G-2 illustrates Metropolitan's IRP process. The process started with a level of service objective (reliability goal) and moved to the identification of resource options (imported supplies, local

supplies, conservation, and capital improvements). After resource options were developed, combinations of these options were grouped to form resource mixes (or strategies) designed to meet the multiple objectives of the IRP. The resource mixes were then evaluated in terms of their reliability, cost and rate impacts, risk, and environmental impacts. The process allowed for some iterative movements back and forth. For example, if the selected resource mix resulted in unacceptable rate increases, then the process would return to the reliability goal for adjustment.

**Figure G-2
Technical IRP Process**



The discussion of supply reliability and IRP extends the technical work found in the power industry (see Wu and Gross, 1979; Booth, 1972; Hirst and Schweitzer, 1988; and Barakat & Chamberlin, 1994). However, the application of probability and simulation analyses and the rigorous evaluation of storage and other means of improving supply reliability represents an innovative and unique approach in the water industry.

NEEDS ASSESSMENT

A critical component to the assessment of supply reliability and development of an IRP is a credible and accurate water demand forecast. Much progress has been made in developing more advanced techniques for forecasting water demands. The use of econometric models that relate water use to major determinants such as housing type, family size, income, lot size, weather, and the price of water are increasing in the water industry. Metropolitan uses a customized version of the IWR-MAIN model which projects residential, commercial and industrial, and public water uses based on econometric models. Although this model does not use the simple per capita water use approach to demand forecasting (multiplying population by an assumed per person water usage factor), the resulting output explains why per capita water

use increases or decreases over time. This ability to explain the effects that several factors have on demand is one of the strongest attributes of the IWR-MAIN model.

The model indicates that about 66 percent of the region's future urban water use will be in the residential sector, 17 percent in the commercial sector, 6 percent in the industrial sector, and the remaining 11 percent in public and other uses. Figure G-3 summarizes the resulting urban per capita water use estimates that were derived from the model. The model was also used to "backcast" demands in order to explain fluctuations in historical per capita use. For example, the large decreases in per capita use in 1977 and 1993 were both caused by drought conservation, economic recession, and wet/cool weather. The decrease in 1983 was due to extreme wet/cool weather. The model projects that normal-weather per capita use (without conservation) would increase in the future due to: (1) more families moving to the hotter and drier climate zones of the service area; (2) a greater standard of living due to a modest increase in income; and (3) employment growth in commercial sectors that use more seasonal water (Planning and Management Consultants Ltd., 1991). Based on the projected effectiveness of water conservation programs, it is anticipated that daily per capita use could be held down to a level of about 195 gallons.

Figure G-3

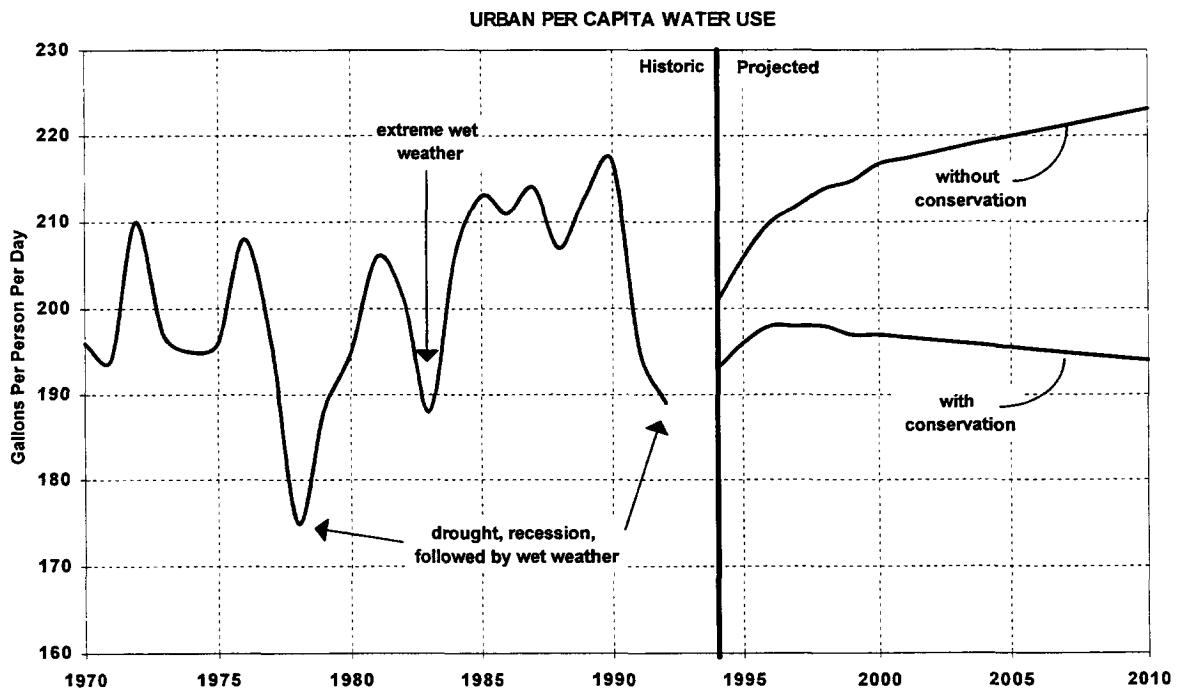
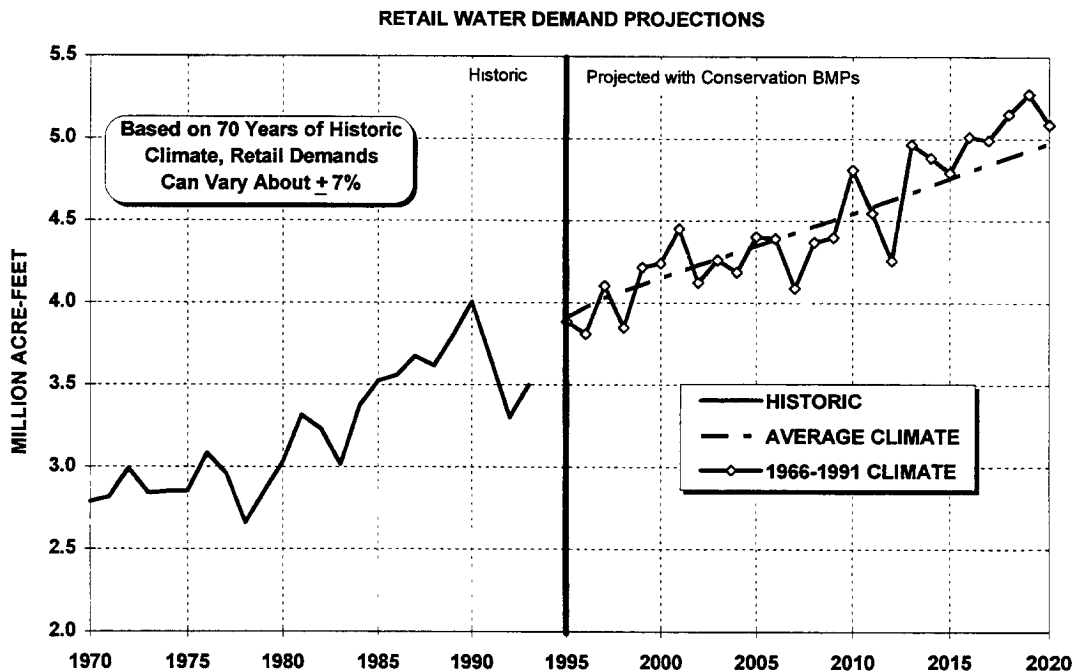


Figure G-4 presents the water demand projections in acre-feet per year, assuming the full implementation of conservation programs. The demand projections are first developed assuming normal weather. However, in order to estimate supply reliability, variations in future demands due to temperature and rainfall must be developed. To illustrate this variation, a climate trace from 1967 to 1991 was superimposed over the future demand projection. Wet and

cool weather would result in lower-than-normal demands, while dry and hot weather would result in greater-than-normal demands. In the historic climate sequence, 1983 (a record wet year) falls on the projection year 2012 -- indicated by the lower-than-average projected demand. The recent six year drought (1986 to 1991) falls on the projection years 2014 to 2020 -- indicated by the greater-than-average projected demands. Based on 70 different historic climate sequences occurring in any given forecast year, the variation due to weather has been estimated to be about ± 7 percent at the 95 percent confidence level.

Figure G-4



In addition to the variations in water demands due to weather, the uncertainty in future demands due to demographic changes, economic growth and forecast error were also included in the reliability analysis. These uncertainties can add another ± 5 percent to the variation in future demands by the year 2020.

RESOURCE ALTERNATIVES

Based on the demand projections and assessment of existing firm water supplies available to the region during a drought, reliability evaluations indicated that about 2.2 million acre-feet of additional water supplies were needed to avoid water shortages that could occur at least 10 percent of the time. The possible local resource alternatives that could be used to meet the anticipated shortfall in supplies include: (1) increasing local groundwater production by storing excess imported water (available during wet and normal weather years) in underground aquifers, and pumping greater amounts of groundwater during dry years -- known as conjunctive use storage; (2) recovering contaminated brackish groundwater by desalination

techniques -- thereby increasing production; and (3) developing reclamation projects that treat wastewater to high quality standards -- such that the water can be used for irrigation, groundwater recharge, and direct industrial uses. Moderate investments in local resource alternatives could produce 0.67 million acre-feet per year of additional supplies by 2020, while large investments could produce 1.10 million acre-feet per year of additional supplies by 2020.

In addition to the local resource options, the IRP identified several imported supply options that could be developed. These imported supply options include: (1) increasing firm supplies from the Colorado River; (2) enhancing supplies from the State Water Project; and (3) voluntary water transfers between willing sellers and buyers. About 1.2 million acre-feet of additional imported supplies could be developed by 2020 with moderate investments, while an additional 2.3 million acre-feet could be developed with large investments.

The IRP also assumed the implementation of long-term water conservation programs which are expected to permanently lower the demand for water into the future. These long-term programs were designed to minimize negative impacts to lifestyle. About 250,000 acre-feet of additional conservation is estimated to occur by year 2000 as a result of plumbing codes and landscape ordinances as well as programmatic demand-side management. By year 2020, it is expected that over 500,000 acre-feet of demand reduction will occur. These estimated savings were based on econometric studies, surveys, plumbing codes, and other studies.

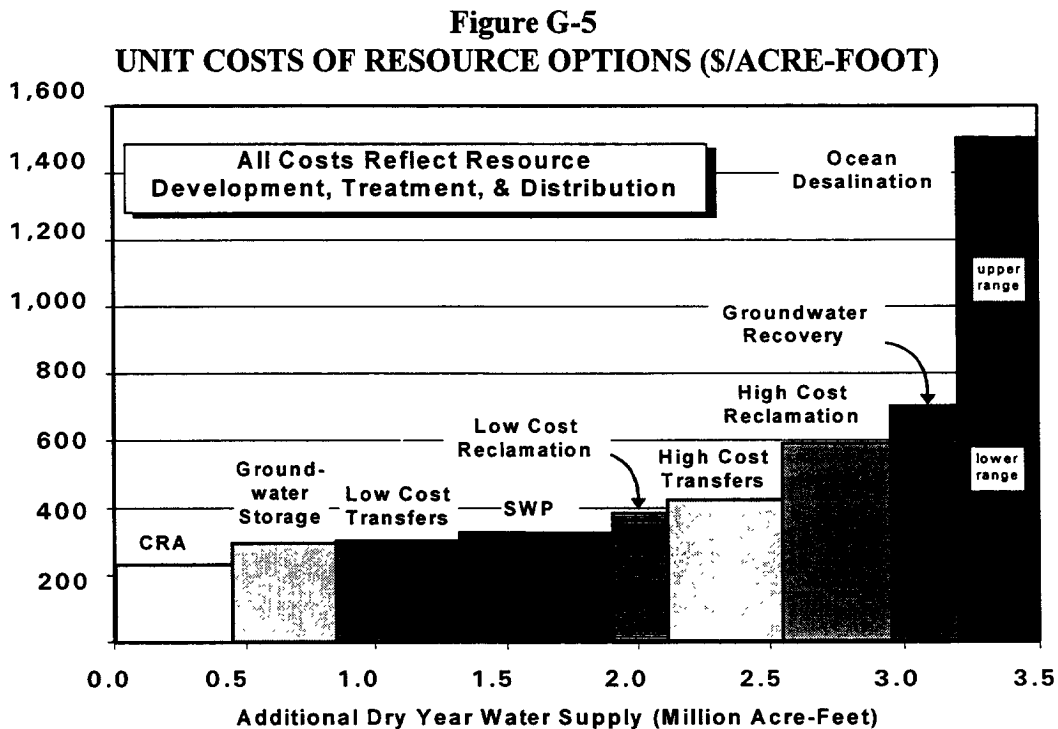
RESOURCE EVALUATIONS

The next step in the IRP process was the grouping of local and imported resource alternatives into resource mixes. The resource mixes were developed and evaluated based on five major objectives:

1. Supply Reliability -- resource alternatives should be grouped such that, when combined, they achieve the desired reliability goal.
2. Cost -- resource alternatives which have the lowest overall unit costs (dollars per acre-foot) should be selected before more expensive options are developed.
3. Water Quality -- impacts to overall water quality need to be considered when selecting the resource alternatives.
4. Flexibility and Diversity -- resource alternatives should be diversified in order to minimize the risks and uncertainties associated with developing the supply or conservation programs.
5. Institutional/Environmental -- institutional and environmental barriers or constraints to resource development should be considered.

Least-Cost Planning

Cost evaluations were based on estimated total project costs (capital and O&M) over the expected life of the project. The costs included developing and acquiring resources, capital investments, and operational and maintenance (O&M) costs for treating, storing, and distributing the supply. Capital costs were assumed to be financed at about 6 percent and future costs were inflated using a 3 to 4 percent annual escalation rate. Constraints were put on the available supply yield from these resource alternatives based on a risk assessment and incorporation of institutional/environmental constraints. The risk assessment and incorporation of institutional and environmental considerations were conducted over a one year period, during which water managers and resource experts were surveyed regarding the likelihood of success of resource development, the potential barriers to development, and means to overcome the barriers. Figure G-5 presents a summary of the unit cost and supply constraints that were used in the evaluations of the resource alternatives.



The graph illustrates that about 3.5 million acre-feet of dry year water supply could be developed over the next 25 years. The resource alternatives are ranked by unit costs (dollars per acre-foot). Unit costs were estimated by taking the capital and O&M costs needed to develop the resources, divided by the anticipated water supply yield over the 25 year planning period. Generally, those resources with the lowest overall unit cost were selected first. However, water quality played an important role in the selection as well. For example, relying on imported water that is not sufficiently blended between Colorado River water (high in salinity content) and State Water Project or water transfers (low in salinity content) could prohibit the development of local resources (reclamation and groundwater storage). This is

due to local groundwater basin water quality standards, and the fact that water high in salinity recycled through reclamation plants will result in extremely low quality water.

Storage Evaluation and Simulation

One of the major differences between the power and water industries is the ability to store water during times of excess (when supplies exceed demand) and to withdraw the water during times of need (when demands exceed supplies). Storage is critical to regions such as Southern California, which sometimes receive heavy rains and snowpack during wet years, yet may go many years between such events. In addition to providing drought benefits, storage also mitigates against catastrophic events such as earthquakes. All of the major imported water supply conveyance systems to Southern California cross the San Andreas Fault, where a major quake is long overdue. But, high costs and potential environmental impacts pose serious problems to developing large surface reservoirs. During the IRP, it became apparent that storing imported water in the large aquifers of the major groundwater basins in Southern California could help achieve the region's storage requirements. To evaluate the benefits of increased storage, a computer model called IRPSIM was developed that accounted for the availability of excess imported supplies, the total storage, the maximum monthly storage (putting water into storage) conveyance, and the maximum monthly withdraw (taking water from storage) conveyance.

An innovative approach called indexed-sequential simulation was used to evaluate the benefits and costs of storage. Indexed simulation means that imported supplies from Northern California and the Colorado River are indexed to the same year as local demand and supplies in Southern California. This methodology preserves the contemporaneous relationships between the hydrology and climate effects on supply and demand. In other words, 1933's weather impact on Northern California's hydrology is matched with 1933's weather impact on demands and local supplies in Southern California and so forth. This indexing between supply and demand is critical because of the relationship between the two. This relationship between supply and demand is another major difference between the power and water industries. Power demands are not necessarily correlated with the variation and uncertainties in power supplies. Outages in power can occur during times of low demand or high demand. Therefore, probability analysis of supply and demand for power reliability can generally be independent of each other. The demand for water, however, is generally correlated with the supply. The same factors that make demand increase (hot and dry weather), also tend to decrease supply availability.

The simulation approach not only preserves the match between supply and demand, but also the sequence of years. Sequential simulation (preserving the order of the historical year's climate and hydrology) can identify the times in which demands exceed supplies and vice versa. This analysis is critical for determining storage needs. In addition, sequential simulation preserves the interrelationship of weather between years. Statistical models that are generally used to generate the weather effect on water demand, or hydrology effect on water supply, measure a multi-year effect. This means that the estimate of 1987's weather effect on demand is, based on the previous two or three year's weather. The same is true for hydrologic models of supply.

Therefore, if 1987 were separated from 1984, 1985 and 1986 in the sequence, then the weather or hydrology effect estimated would not be valid.

Figure G-6 presents a simplified example of an indexed-sequential simulation, where 1967 to 1991 historical weather is mapped over a 1995 to 2020 projection of supplies and demand. The example summarizes the data into annual demands and supplies, and indicates the years in which shortages and surplus exist.

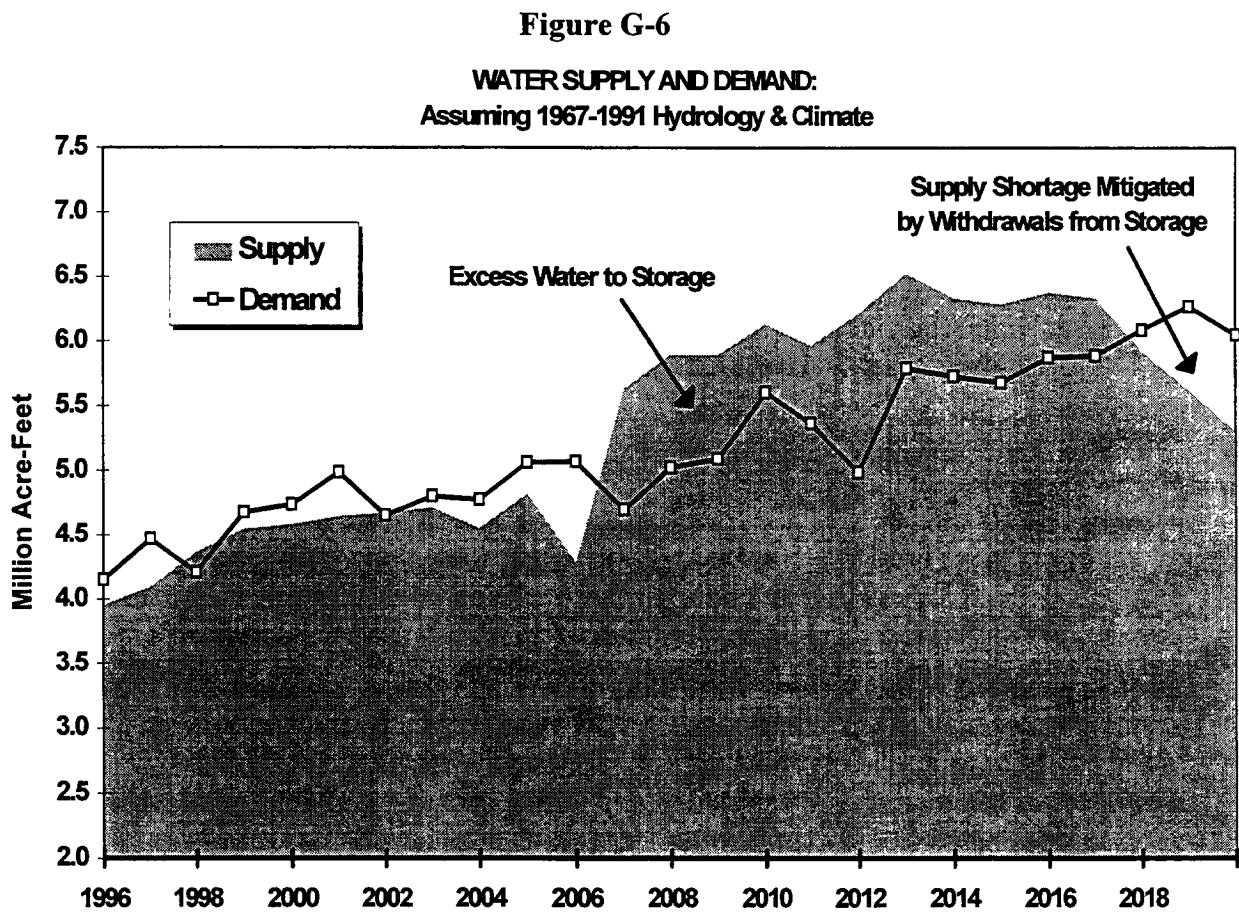
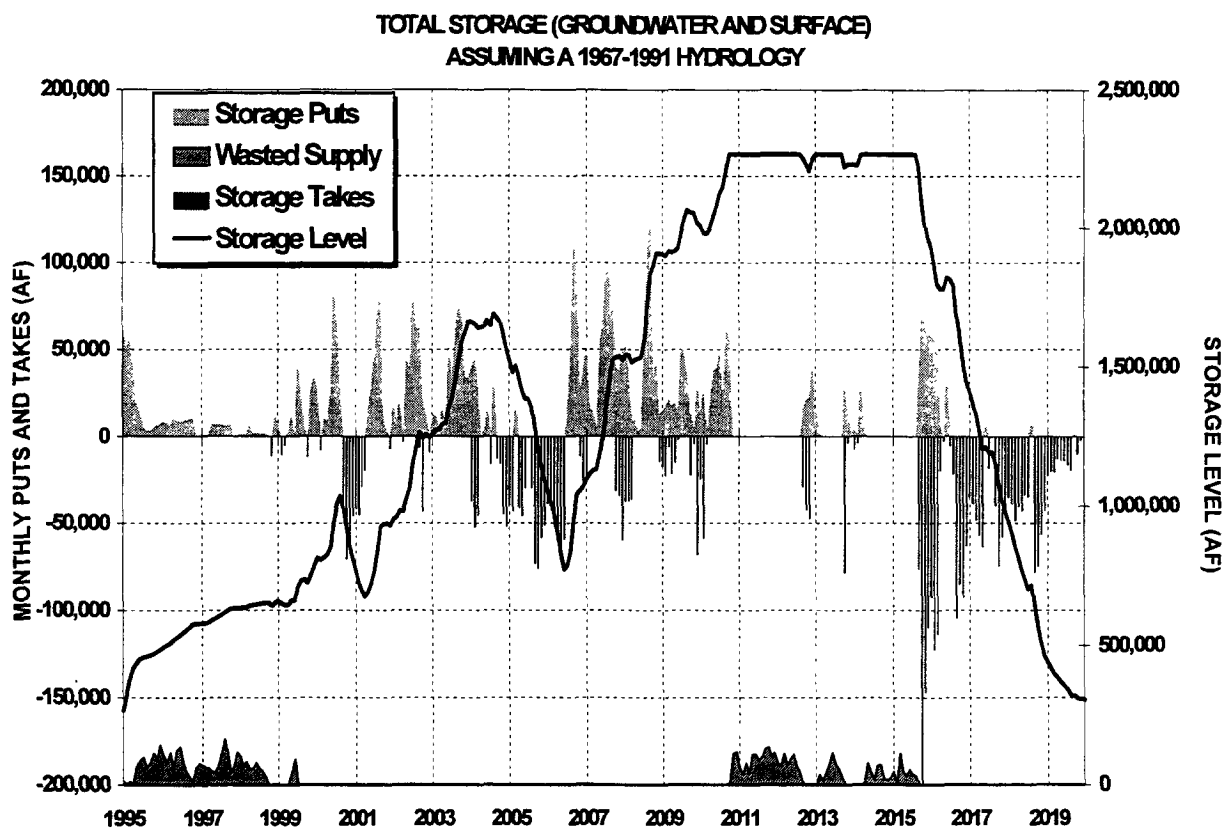


Figure G-7 presents the monthly simulation of storage assuming 1967-1991 historical hydrology and weather. The total storage level is measured by the solid black line, read from the right-hand vertical axis (ranging from 0 to 2.25 million acre-feet). The monthly puts into storage are measured by the light gray shaded area, read from the top portion of the left-hand vertical axis (ranging from 0 to +200,000 acre-feet). The monthly draws from storage are measured by the dark gray bars hanging down, read from the bottom portion of the left-hand vertical axis (ranging from 0 to -200,000 acre-feet). Finally, imported water which is available but cannot be stored (wasted supply) is shown as a gray-hatched shaded area at the bottom of the chart, read from the right-hand vertical axis.

Figure G-7



This particular 1967-1991 weather trace starts off wet, and imported water is stored as fast as the storage capacity can will allow. In the earlier years (before year 2000), only the groundwater basins provide significant storage potential. Because the physical spreading capabilities of the groundwater basins limit the storing of water, available imported water during this period is not fully used. After 1999, the Domenigoni Valley Reservoir Project (a planned 800,000 acre-foot surface reservoir) will be operational to store water for emergency and drought protection for the region. With its large monthly capacity for storing water, the slope of the total storage level increases dramatically and very little available imported water during wet years is unused. The 1976-77 drought (one of the worst on record) occurs in the 2005-06 projection year, as indicated by the heavy withdrawals from storage. The total storage level falls from 1.70 million acre-feet to about 0.70 million acre-feet in two years. The period following the 1976-77 drought was very wet and cool, allowing water to be quickly stored. Finally, the worst drought on record (1986 to 1991) occurs in the projection period of 2015 to 2020. This multi-year drought draws down the total storage level from 2.25 million acre-feet down to the emergency reserves of about 400,000 acre-feet over a five year period. This example represents only one such weather trace with a given demand growth. The storage benefits were evaluated using 70 historical weather traces and about 28 different demand scenarios.

SUPPLY RELIABILITY EVALUATION

In general, water supply reliability can be defined as: *the degree to which the performance of a supply system results in the delivery of water service to its customers in the amounts desired, within acceptable quality standards.* Evaluation of supply reliability is important because it provides a signal when additional resources and capital investments are required. Equally important, reliability planning determines when "enough is enough" -- that is, when additional resources or capital planning would constitute an over-investment in supply.

Supply reliability was measured using IRPSIM, an indexed-sequential and Monte-Carlo simulation computer model (Chesnutt and McSpadden, 1994). Supply reliability measures the likelihood and magnitude of supply shortages (when demand exceeds supply) and supply surplus (when supply exceeds demand). Supply reliability has major two components: (1) frequency -- how often does the supply shortage or surplus occur; and (2) magnitude -- how large is the supply shortage or surplus. Typically, reliability planning focuses on the shortage aspect, but it is also important to understand the surplus side of the equation. As discussed earlier, identification of surplus water supply conditions are critical for the evaluation of storage. Evaluation of surplus conditions also reveals the effectiveness of water supply and management investments.

Reliability Measurement

Measuring supply reliability can involve a great deal of analytical effort. Traditional methods of reliability analysis, borrowed from the power industry, were used as the basis for the analyses in the IRP. However, because power is not economically storable, the reliability evaluations had to be adapted for water. The simplest model for evaluating supply reliability in the power industry starts by estimating mean future demands and its potential distribution. A statistical demand model can have many predictors such as demographics, time of the year, and weather. However, even the best statistical predictions have remaining uncertainty or error.

Supply models also contain forecasting error and it is this combination of the variations in supply and demand that are used to estimate supply reliability. However, the distributions and interrelationships of supply and demand variables are often too difficult to derive by pure mathematical means. In order to avoid dealing with this computational problem, Monte Carlo simulation was used. By making random draws from distributions and mathematically manipulating them, a new distribution can be formed. In this way, distributions can be created one observation at a time without ever having to explicitly derive the mathematical formula for the new distribution.

The Monte-Carlo methods developed for IRPSIM are best described in their simplest form. Assume water supply and demand were independent normal distributions (see Figure G-8a). Simply by taking a random draw from each distribution and subtracting them (supply minus demand), and repeating this hundreds of times, a distribution (see Figure G-8b) of shortage/surplus can be derived.

However, this method is complicated by the negative correlation between supply and demand (see Figure G-9). For example, the same conditions that make demand increase (hot and dry weather), also tend to make supplies decrease.

Figure G-8a

Probability Distributions of Water Supply and Demand

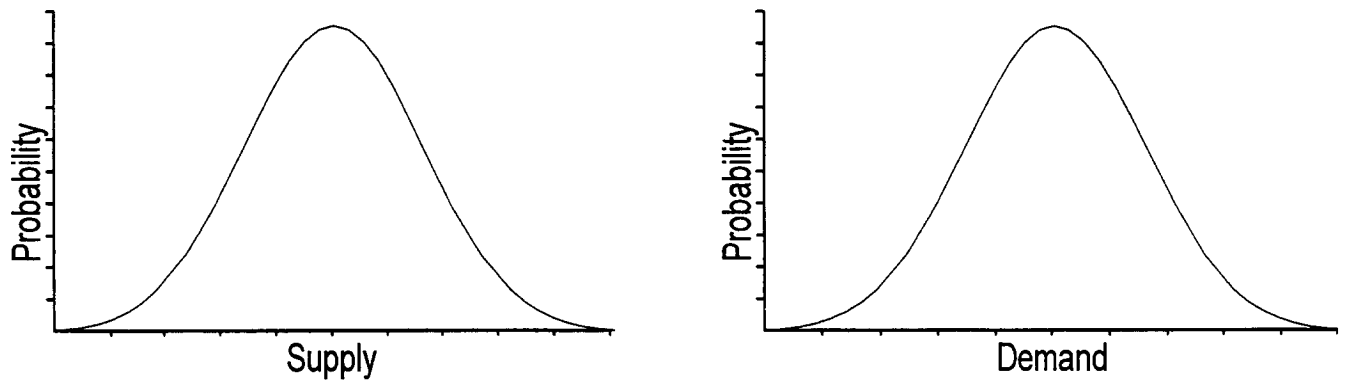


Figure G-8b

Probability Distribution of Water Supply Less Demand

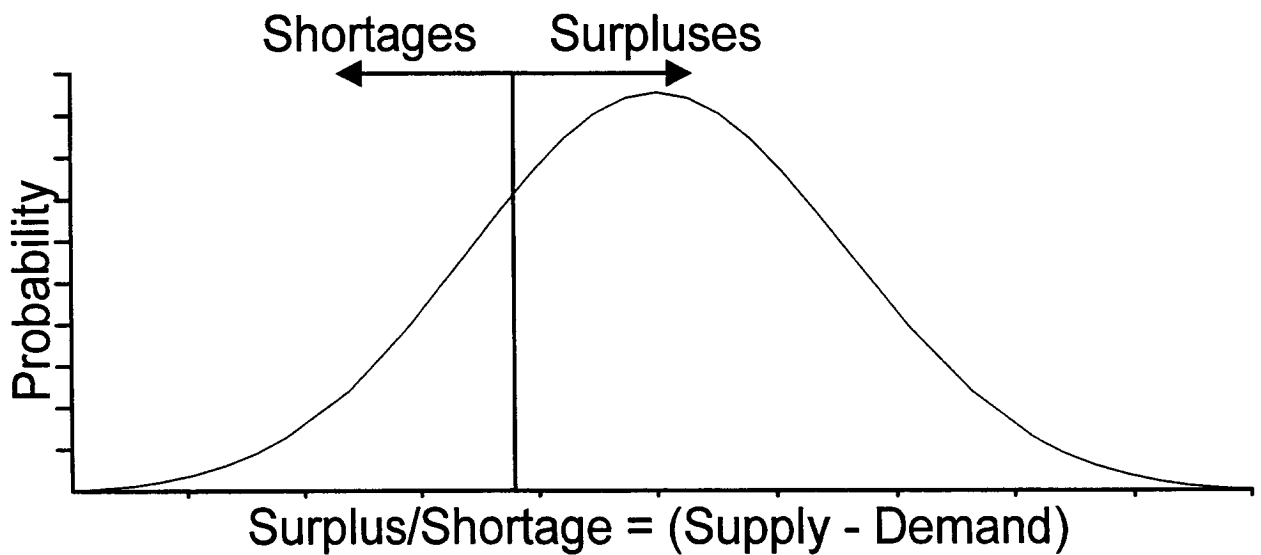
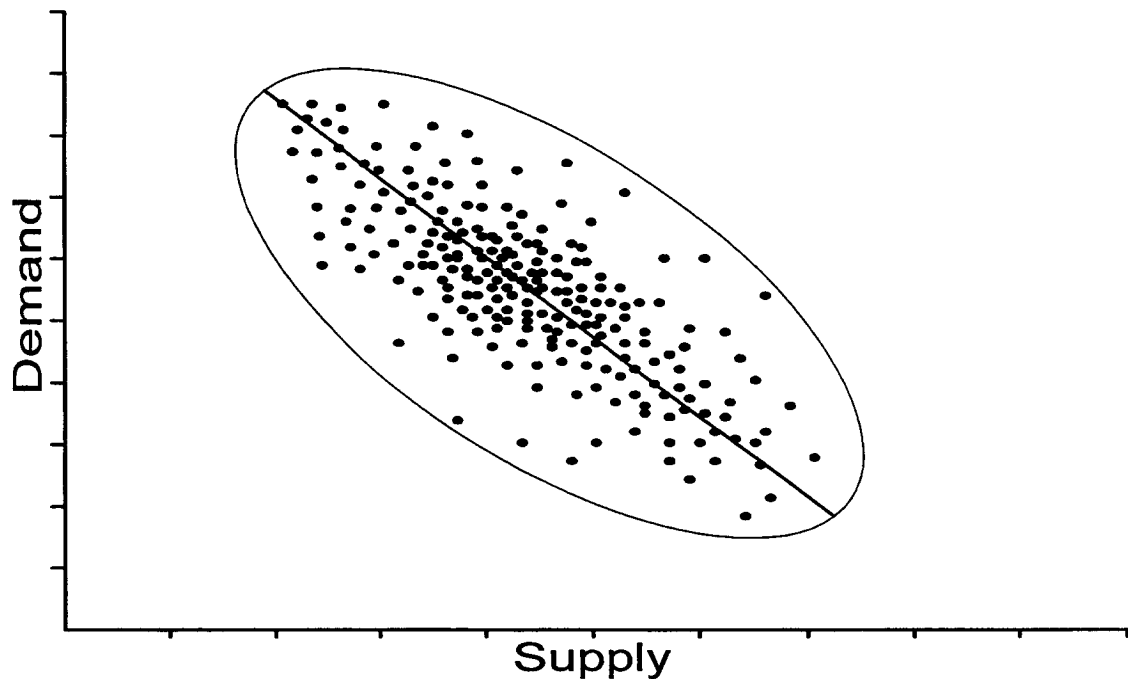


Figure G-9
Relationship Between Supply and Demand



Therefore, in order to determine supply reliability for water, matched pairs of supply and demand must be used to develop the distribution of supply less demand. In other words, there is a low likelihood that a low demand observation gets paired with a low supply observation. IRPSIM combines the indexed-sequential simulation discussed earlier with Monte-Carlo probability analysis in order to obtain the final distribution used to estimate supply reliability. The model takes each of the unique 70 year climate/hydrology traces (from 1922-1991) and draws about 28 different random non-weather related demands. This provides about 2,000 individual events for any specified time-step (usually monthly).

In order to estimate a reliability curve for any given time period, the distribution of supply less demand should not be displayed as a probability density function but as a cumulative probability distribution, by integrating the curve (see Figure G-10a). In this form, the probability of shortage or surplus can be read directly from the graph. But for further ease, this graphic can be rotated 90 degrees counter clockwise (see Figure G-10b). Now the likelihood (or frequency) of shortage or surplus is read on the horizontal axis and the magnitude of shortage or surplus is read on the vertical axis.

Figure G-10a
Cumulative Probability of Supply Shortage and Surplus

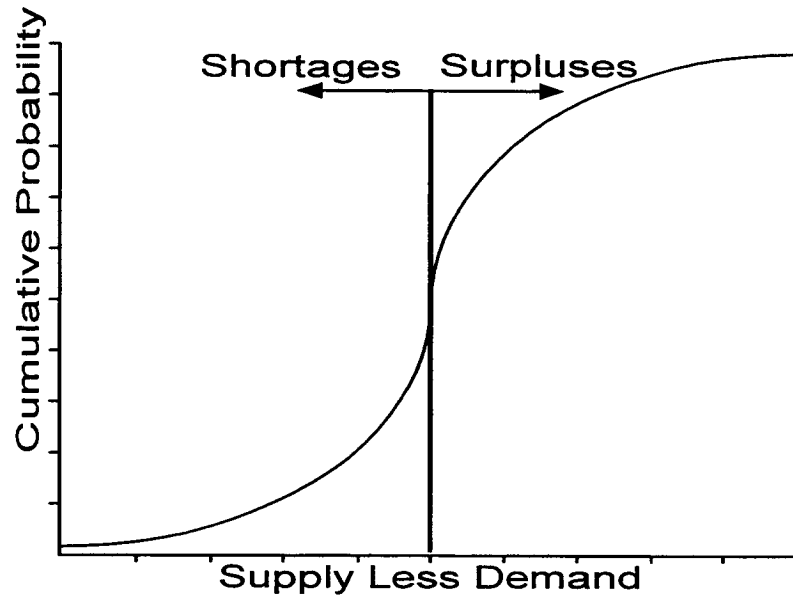
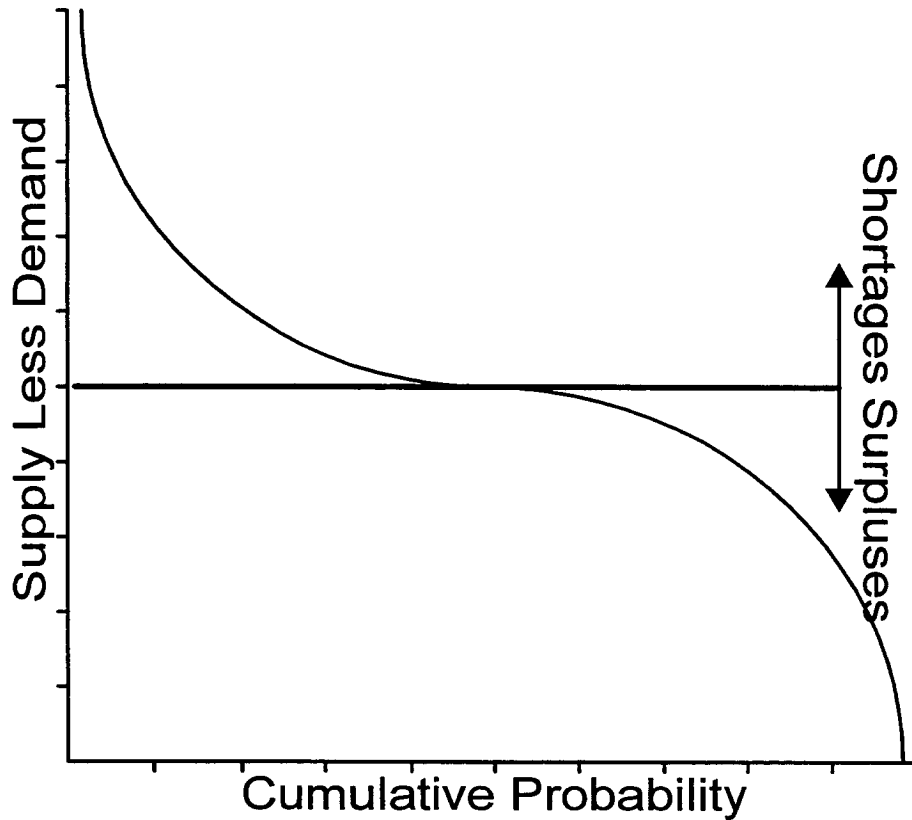


Figure G-10b
Rotated Cumulative Probability of Supply Shortage and Surplus



This example is greatly simplified because it does not include the impact of storage. To understand the impact of storage, it is instructive to illustrate how the reliability curve is affected by different supply enhancements. Supply reliability can be improved from basically three different types of water resource enhancements (or investments):

1. Core Supply -- investments are made for year round supply, whether they are needed in every year or not. Core investments *decrease* the likelihood and magnitude of water shortages, but at the same time *increase* the likelihood and magnitude of water surplus. Since capital expenditures do not vary with water supply yield, a portion of the core supply's cost will remain fixed even if the supply is not needed. For this reason, core supplies can be relatively expensive during wet years and normal years.
2. Storage -- investments are made to store excess water during times of plenty for use during times of need. Storage investments *decrease* the likelihood and magnitude of shortages and also *decrease* the likelihood and magnitude of surplus -- because they transfer surplus water to meet shortages. Storage investments may have relatively high unit costs in terms of total yield (because the supply yield is only used periodically), but may be cheaper than core supplies over the long term.
3. Swing Supply -- investments are made for water only when needed, such as option or spot market water transfers. These investments only *decrease* shortages and do not affect the frequency or magnitude of surplus water. Even if the dry year unit costs are higher than core supplies or storage, the average costs over time will likely be lower -- because the costs are paid only when the supply is used. However, flexible supplies can have a higher degree of uncertainty than core supplies or storage.

The following discussion illustrates how different water resource investments affect supply reliability. A core supply improvement (such as a reclamation facility) shifts the entire reliability curve downward (see Figure G-11a), because the supply is available under all hydrologic conditions. This can also be displayed as a shift to the right on the supply distribution curve (see Figure G-11b).

The evaluation of storage requires an evaluation of the raw reliability curve (see Figure G-11a) and the determination of a surplus or shortage condition. Based on this condition, water is either placed into or drawn from storage effectively reducing shortages and reducing surplus (see Figure G-12a). It also collapses the supply distribution from either side (Figure G-12b). Although the collapse of the supply distribution appears uniform in this example, the collapse is more likely to be skewed in either the right (if production capacity is less than storage capacity) or to the left (if storage capacity is less than production capacity). Only if storage operations were perfect (the same amount of water going into storage comes out of storage) would the collapse of the distribution curve be uniform.

Figure G-11a
Core Supply Improvement to the Supply Reliability Curve

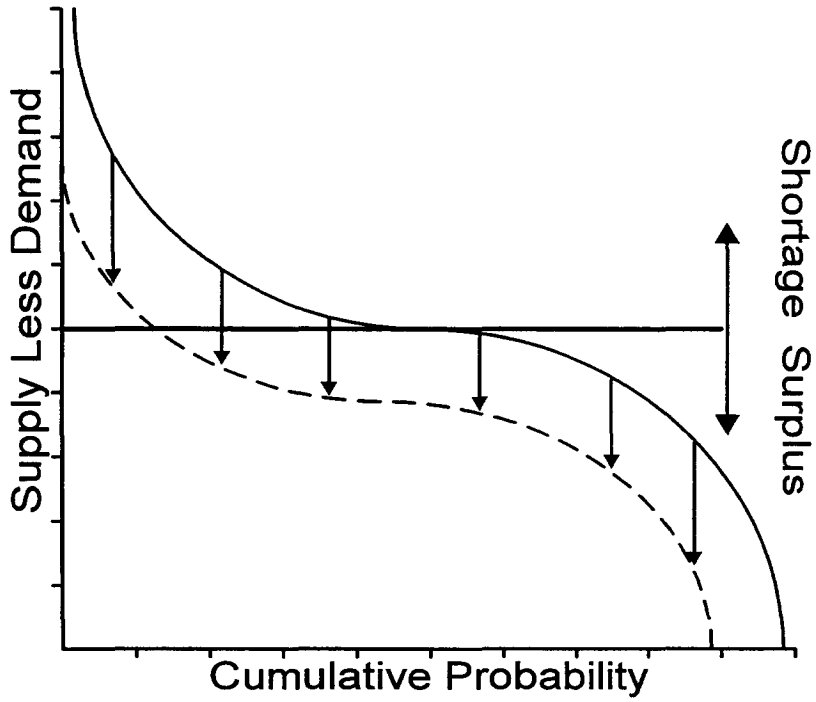


Figure G-11b
Core Supply Improvement to the Supply Distribution Curve

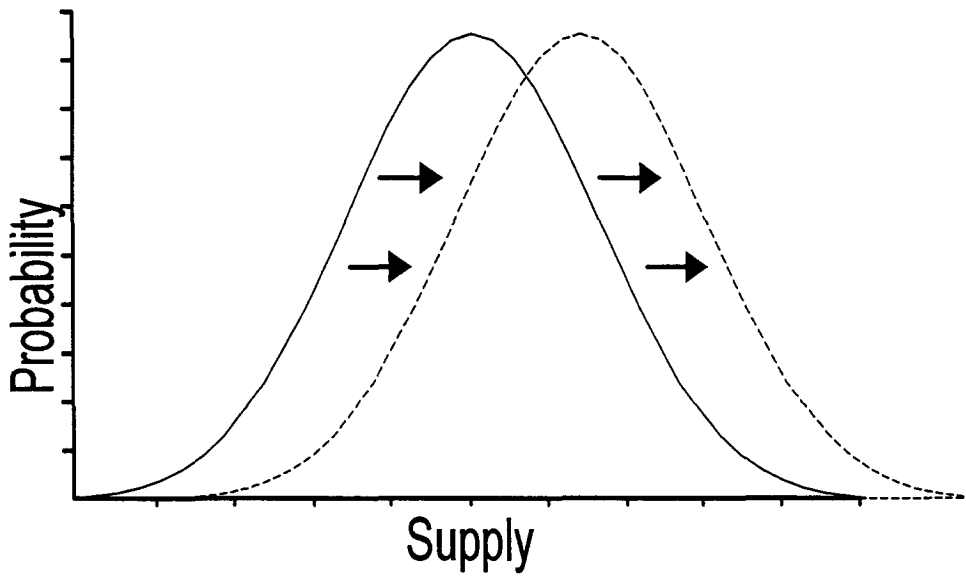


Figure G-12a
Storage Improvement to the Supply Reliability Curve

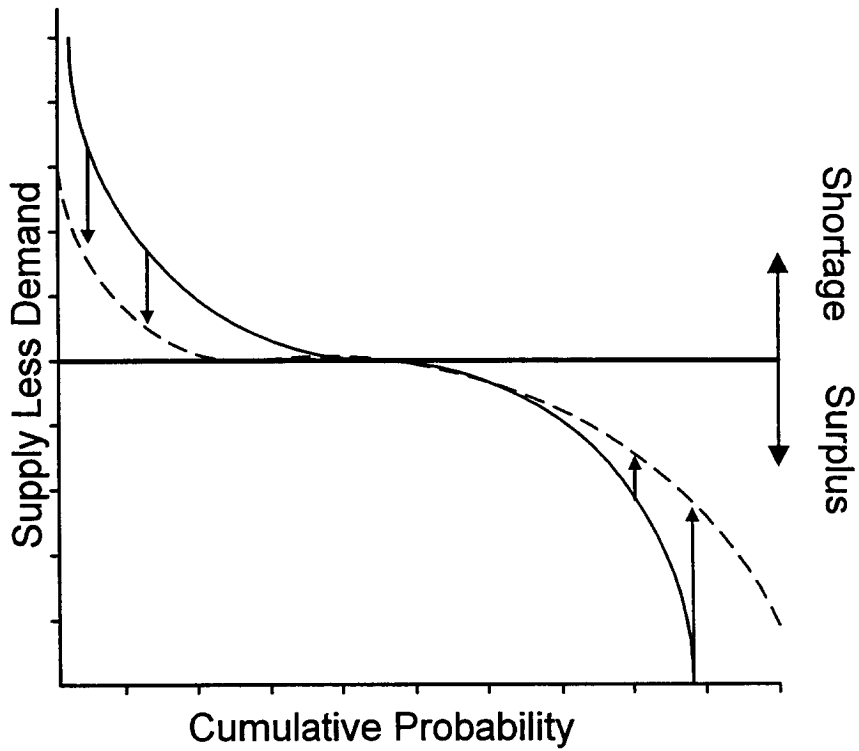
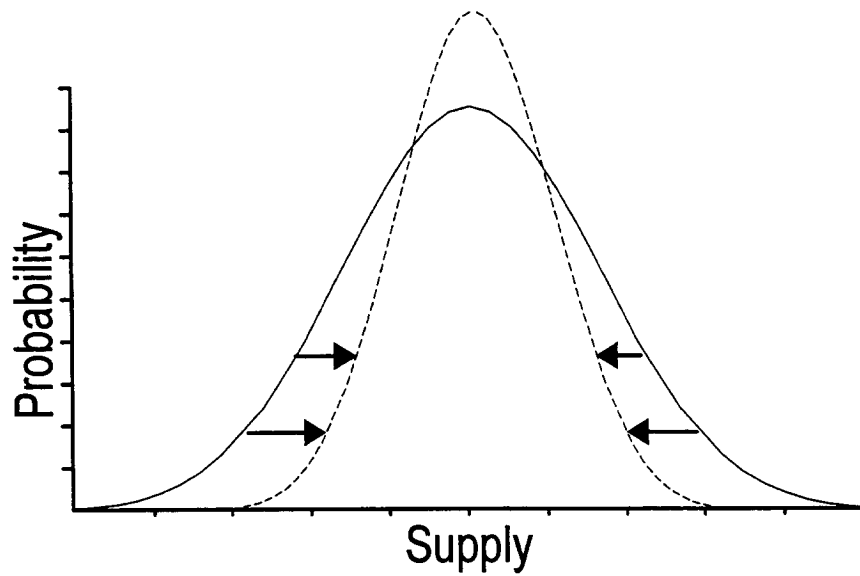


Figure G-12b
Storage Improvement to the Supply Distribution Curve



The actual measurement of the potential for storage to increase reliability depends on the inter-temporal nature of storage. The ability to put to or take from storage is dependent on the total storage capacity, conveyance constraints, availability of excess water, and the remaining storage capacity (or level) from the prior time period. Although theoretical models have been developed to predict weather in the short-term, no long-term forecast models have been used successfully. Because of this fact, the simulation used to evaluate supply reliability should maintain the sequence of the historical weather and hydrology.

Flexible supplies, such as water transfers, are used to help mitigate supply shortages. The augmentation of supply only occurs during the shortage, and for this reason, the supply curve is only shifted downward for the shortage, not the surplus (see Figure G-13a). The supply distribution is skewed rather than shifted as a result of a flexible supply (see Figure G-13b).

In reality, a diverse mix of core supplies, storage, and flexible supplies should be pursued. Based on detailed evaluations of different resource options, a diversified approach will tend to minimize overall costs, reduce wasted supply, and lower the overall risk in supply development. This notion of diversification of resources is consistent with the literature and studies conducted in the power industry (Hall and Thomas, 1984).

Figure G-14 presents an estimate of the retail level supply reliability for Metropolitan's service area in the year 2020 using the techniques described in this paper. The resource mix evaluated is a combination of cost effective local water supplies (reclamation, conservation, and groundwater), surface and groundwater storage, improvements to imported supply, and voluntary water transfers.

The top half of the graph depicts supply shortages, with the likelihood of shortages read from the top. The top portion of the left-hand axis measures the percent of full service retail demand that would not be met. For example, the reliability curves indicate that without future investments in supplies, shortages of about 30 percent could occur about 10 percent of the time. With core supply improvements, the shortages would be reduced to 15 percent, occurring about 10 percent of the time. Finally, with storage improvements, the shortages are further reduced to under 10 percent, occurring 10 percent of the time. The bottom half of the graph measures the likelihood and magnitude of supply surplus. No supply surplus would occur if no future investments are made by year 2020 (in other words, there is a 100 percent chance that some kind of water shortage would exist). When core supply investments are made, the shortages are reduced, but the surplus is about 10 percent, occurring 10 percent of the time. Storage reduces the surplus to about 5 percent, occurring 10 percent of the time.

Figure G-13a
Flexible Supply Improvement to the Supply Reliability Curve

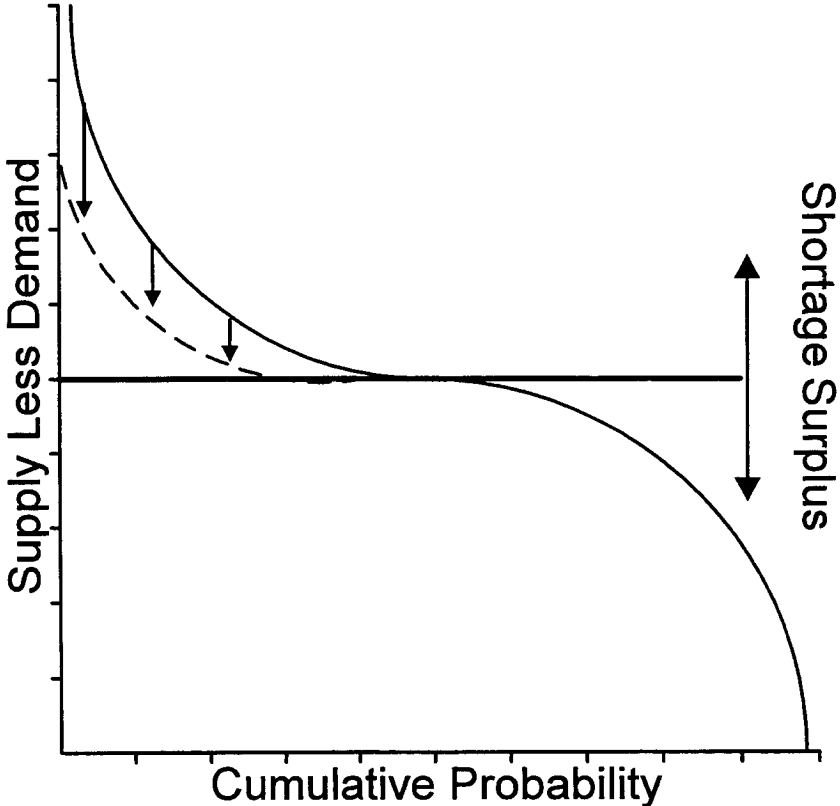


Figure G-13b
Flexible Supply Improvement to the Supply Distribution Curve

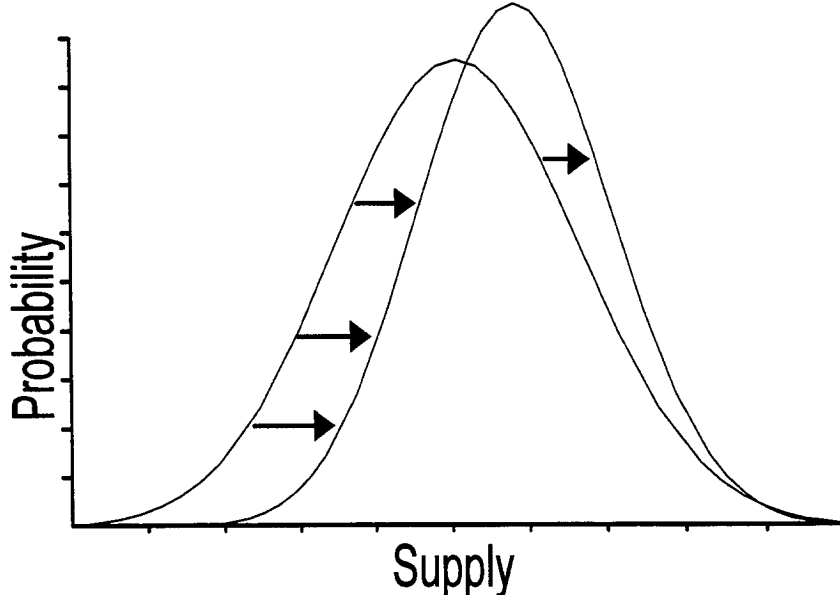
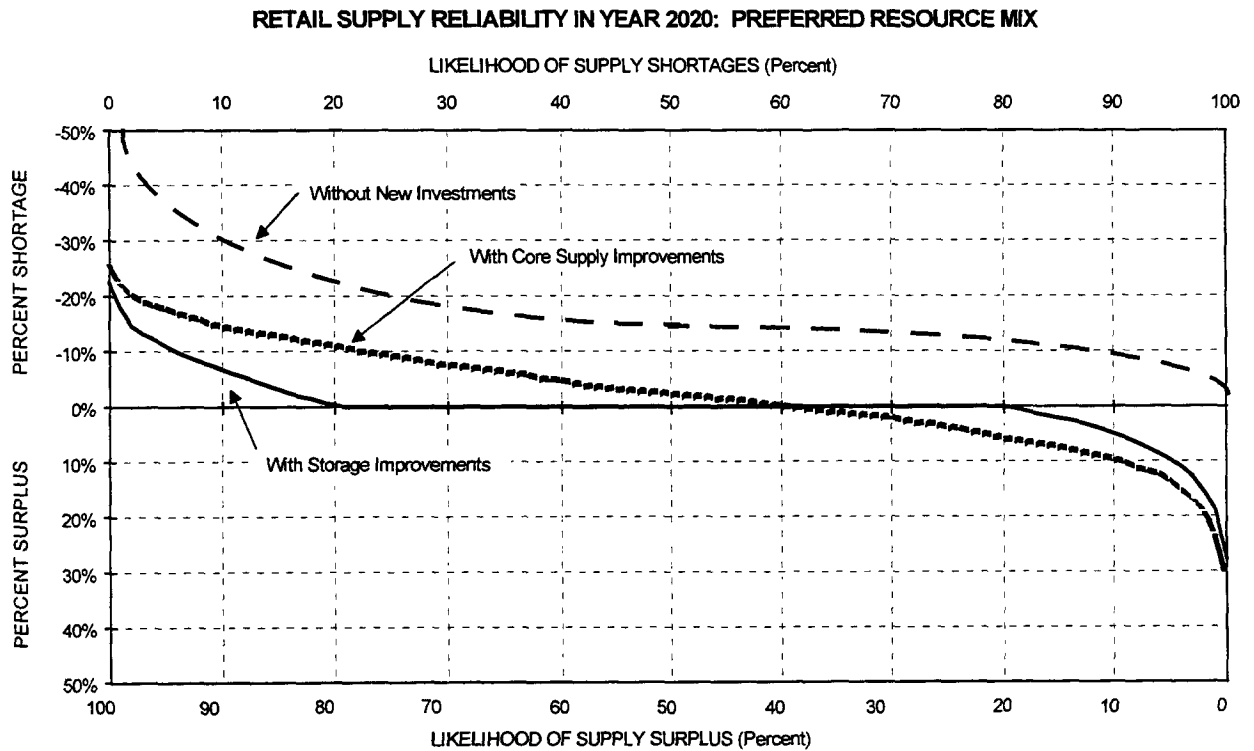


Figure G-14



Metropolitan’s wholesale supply reliability goal, translated into a retail goal, would imply that no shortage should be allowable 90 percent of the time, and that the maximum magnitude of the shortage should be less than 10 percent of full service retail demand. Although this evaluation indicated that the reliability goal could not be achieved with just core supply and storage improvements, water transfers could be used as a cost-effective supply to completely eliminate the remaining shortages. Based on the reliability evaluation, about 400,000 acre-feet of Central Valley water transfers would be needed about 10 percent of the time.

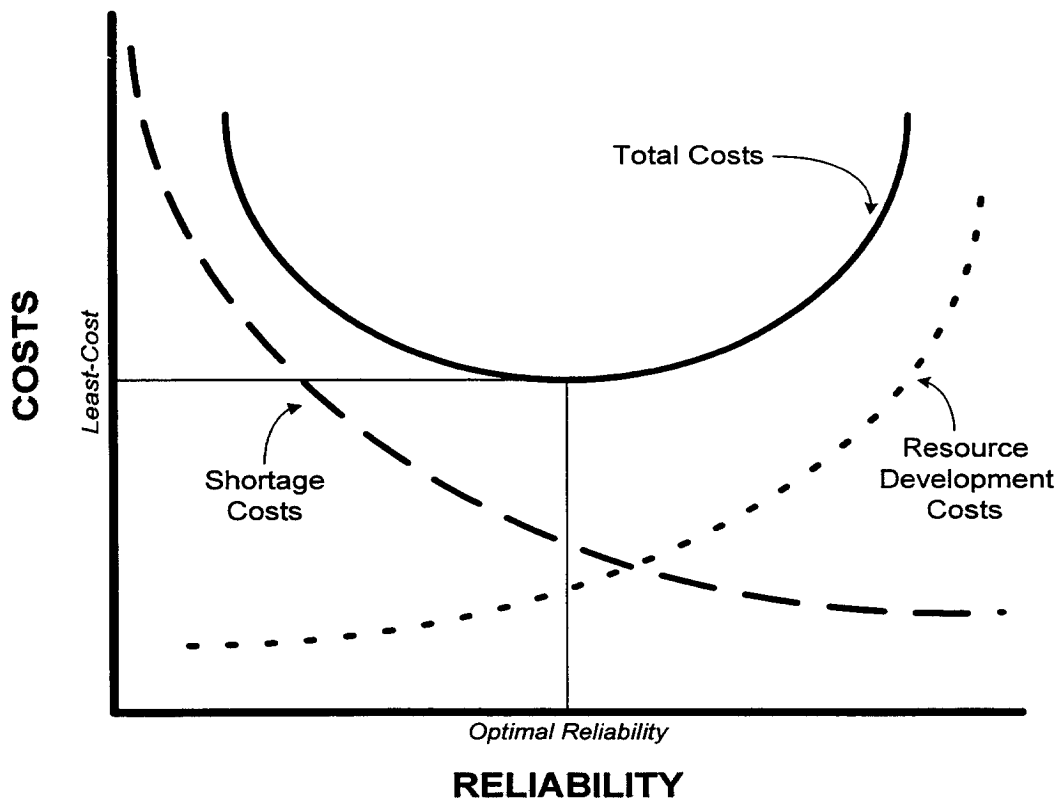
Costs and Benefits of Supply Reliability

The costs and benefits associated with supply development can also be determined by an extensive supply reliability evaluation. Ideally, the optimal level of reliability should be set to minimize total costs. Total costs should include all costs related to developing, treating, storing and distributing water, plus any environmental costs of development. The total costs should also include the adverse impacts to the region’s economy and lifestyle that could occur if chronic water shortages exist. Figure G-15 presents a theoretical approach to setting the appropriate reliability.

The graph indicates that as reliability improves, the costs of resource development increase. If reliability decreases, the shortage costs (negative impacts to the economy and lifestyle)

increase. The sum of these two cost curves (resource development and shortage costs) yields a total cost curve -- where optimal reliability is at the minimum point of the curve. In most cases, the construction of perfect cost curves will not be possible. Although resource development costs may be fairly easy to obtain for different levels of reliability, cost expenditures in the water industry are typically disjointed and "lumpy," rather than smooth curves.

Figure G-15
Least-Cost Reliability Planning



On the other hand, obtaining shortage costs for different levels of reliability is much more difficult. Measurement of the adverse impacts to the economy due to chronic water shortages can be obtained by examining actual case studies, but transference of the results may not be accurate. Statistical and economic input/output studies have been used to estimate the potential impact of supply shortages in the water sensitive manufacturing sector for California and can be helpful. Based on such studies, it has been estimated that a 15 percent shortage to the water sensitive industries in Southern California could cause about \$3.5 to \$4.3 billion in lost jobs and production (Spectrum Economics, 1991). However, most city councils and water boards are unlikely to short large commercial and industrial water customers for the fear of reducing economic output. Therefore, it is the residential customer that will most likely do without during shortages.

One way to measure impacts to residential users is by estimating their willingness to pay for decreased supply shortages. This can be done using contingent valuation analyses. This approach uses detailed surveys to determine willingness to pay for services that are typically difficult to measure (such as recreation, environmental protection, and resource reliability). Contingent valuation surveys completed in Southern California indicated that residential customers were, on average, willing to pay an additional \$10 to \$15 more per month in order to avoid varying levels of water shortages (Barakat & Chamberlin Inc., 1994).

Based on the results of the reliability evaluation, the costs of achieving the reliability goal specified in Metropolitan's IRP were estimated. These costs would result in a \$3 to \$5 increase in the average monthly water bill over the next 10 years for the region. Based on the economic studies and surveys of industry and residential water customers concerning supply shortages (as noted above), the costs for improved reliability are well below the costs associated with the chronic supply shortages that would exist without the new investments.

REFERENCES

- Barakat & Chamberlin Inc., *The Value of Water Supply Reliability: Results of a Contingent Valuation Survey of Residential Customers*. Sacramento, California: California Urban Water Agencies, August 1994.
- Beecher, J. A., *Integrated Water Resource Planning: Discussion Paper*. Sponsored by the Water Industry Technical Action Fund. Columbus, Ohio: The National Regulatory Research Institute, December 1992.
- Booth, R., "Power System Simulation Model Based on Probability Analysis." *IEEE Transactions*, PAS-91, 1:62-69, 1972.
- Chesnutt, T. W. and C. N. McSpadden, *Putting the Pieces Together: Decision Support for Integrated Resources Planning Using IRPSIM*. Washington D.C.: A&N Technical Services, April 1994.
- Hall, D. C. and B. G. Thomas, "Investment in Alternative Electric Supply Plans by Publicly Owned Utilities" *Resources and Energy*, Vol. 6 No. 2 pp. 165-186, June 1984.
- Hirst, E. and M. Schweitzer, *Uncertainty in Long-term Resource Planning for Electric Utilities*. Oak Ridge, TN: Oak Ridge National Laboratory, December 1988.
- Metropolitan Water District of Southern California, *Integrated Resources Plan Assembly - Assembly Statement*. Los Angeles, California, June 1994 and March 1995.
- Planning and Management Consultants Ltd., *Municipal and Industrial Water Use in the Metropolitan Water District Service Area: Interim Report No. 4*. Carbondale, Illinois, June 1991.
- Spectrum Economics, *Cost of Industrial Water Shortages*. Sacramento, California: California Urban Water Agencies, August 1994.
- Wu, F. and G. Gross, *Probabilistic Simulation of Power System Operation for Production Cost and Reliability Evaluation*. Department of EECS and ERL, University of California - Berkeley and Pacific Gas & Electric, 1979.